

Solid Mechanics (plane stress/strain)

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- **Stiffness matrix of Turner triangle**
- **2D plane stress model**
 - ✓ **Kirsh's problem**
- **Quarter model**

PLANE STRESS

PLANE STRESS

The plane stress variant of the 2D interface is useful for analyzing thin in-plane loaded plates. For a state of plane stress, the out-of-plane components of the stress tensor are zero.

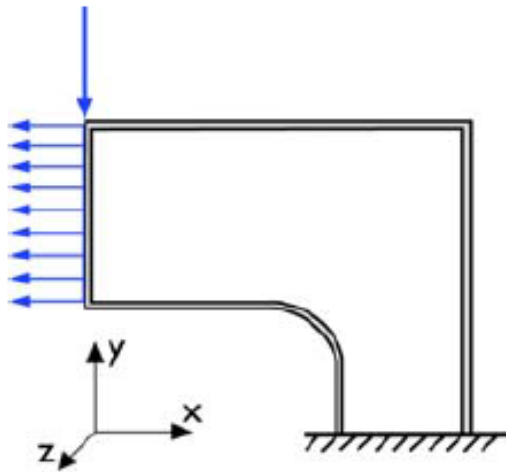


Figure 14-2: Plane stress models plates where the loads are only in the plane; it does not include any out-of-plane stress components.

The 2D interface for plane stress allows loads in the x and y directions, and it assumes that these are constant throughout the material's thickness, which can vary with x and y . The plane stress condition prevails in a thin flat plate in the xy -plane loaded only in its own plane and without any z direction restraint.

PLANE STRAIN

PLANE STRAIN

The plane strain variant of the 2D interface that assumes that all out-of-plane strain components of the total strain ϵ_z , ϵ_{yz} , and ϵ_{xz} are zero.

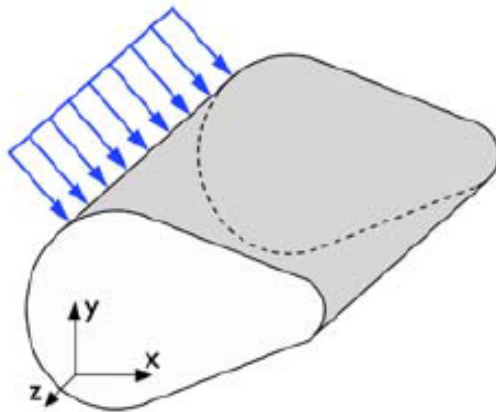


Figure 14-3: A geometry suitable for plane strain analysis.

Loads in the x and y directions are allowed. The loads are assumed to be constant throughout the thickness of the material, but the thickness can vary with x and y . The plane strain condition prevails in geometries, whose extent is large in the z direction compared to in the x and y directions, or when the z displacement is in some way restricted. One example is a long tunnel along the z -axis where it is sufficient to study a unit-depth slice in the xy -plane.

RELATION OF STRESS/STRAIN

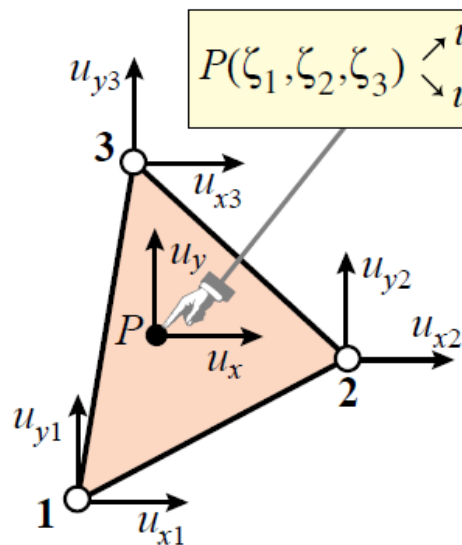
$$\left\{ \begin{array}{l} \varepsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)] \\ \varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_z + \sigma_x)] \\ \varepsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)] \end{array} \right\} \xleftrightarrow{G = \frac{E}{2(1+\nu)}} \left\{ \begin{array}{l} \gamma_{xy} = \frac{1}{G} \tau_{xy} \\ \gamma_{yz} = \frac{1}{G} \tau_{yz} \\ \gamma_{zx} = \frac{1}{G} \tau_{zx} \end{array} \right.$$

$$\text{plane stress } (\sigma_{zz} = \sigma_{xz} = \sigma_{yz} = 0): \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{bmatrix}$$

$$\text{plane strain } (e_{zz} = e_{xz} = e_{yz} = 0): \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \frac{E}{(1+\nu)(2-\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1}{2}(1-2\nu) \end{bmatrix} \begin{bmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{bmatrix}$$

- Stiffness matrix of Turner triangle
- 2D plane stress model
 - ✓ Kirsh's problem
- Quarter model

TURNER TRIANGLE (1)



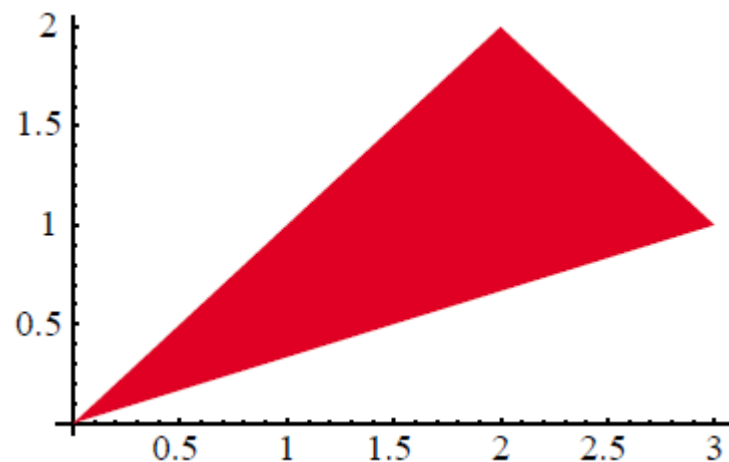
$$u_x = u_{x1}\zeta_1 + u_{x2}\zeta_2 + u_{x3}\zeta_3, \quad u_y = u_{y1}\zeta_1 + u_{y2}\zeta_2 + u_{y3}\zeta_3.$$

$$\mathbf{e} = \mathbf{D} \mathbf{N} \mathbf{u}^e = \frac{1}{2A} \begin{bmatrix} y_{23} & 0 & y_{31} & 0 & y_{12} & 0 \\ 0 & x_{32} & 0 & x_{13} & 0 & x_{21} \\ x_{32} & y_{23} & x_{13} & y_{31} & x_{21} & y_{12} \end{bmatrix} \begin{bmatrix} u_{x1} \\ u_{y1} \\ u_{x2} \\ u_{y2} \\ u_{x3} \\ u_{y3} \end{bmatrix} = \mathbf{B} \mathbf{u}^e$$

$$\boldsymbol{\sigma} = \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{12} & E_{22} & E_{23} \\ E_{13} & E_{23} & E_{33} \end{bmatrix} \begin{bmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{bmatrix} = \mathbf{E} \mathbf{e}$$

$$\mathbf{K}^e = A h \mathbf{B}^T \mathbf{E} \mathbf{B} = \frac{h}{4A} \begin{bmatrix} y_{23} & 0 & x_{32} \\ 0 & x_{32} & y_{23} \\ y_{31} & 0 & x_{13} \\ 0 & x_{13} & y_{31} \\ y_{12} & 0 & x_{21} \\ 0 & x_{21} & y_{12} \end{bmatrix} \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{12} & E_{22} & E_{23} \\ E_{13} & E_{23} & E_{33} \end{bmatrix} \begin{bmatrix} y_{23} & 0 & y_{31} & 0 & y_{12} & 0 \\ 0 & x_{32} & 0 & x_{13} & 0 & x_{21} \\ x_{32} & y_{23} & x_{13} & y_{31} & x_{21} & y_{12} \end{bmatrix}.$$

TURNER TRIANGLE (2)



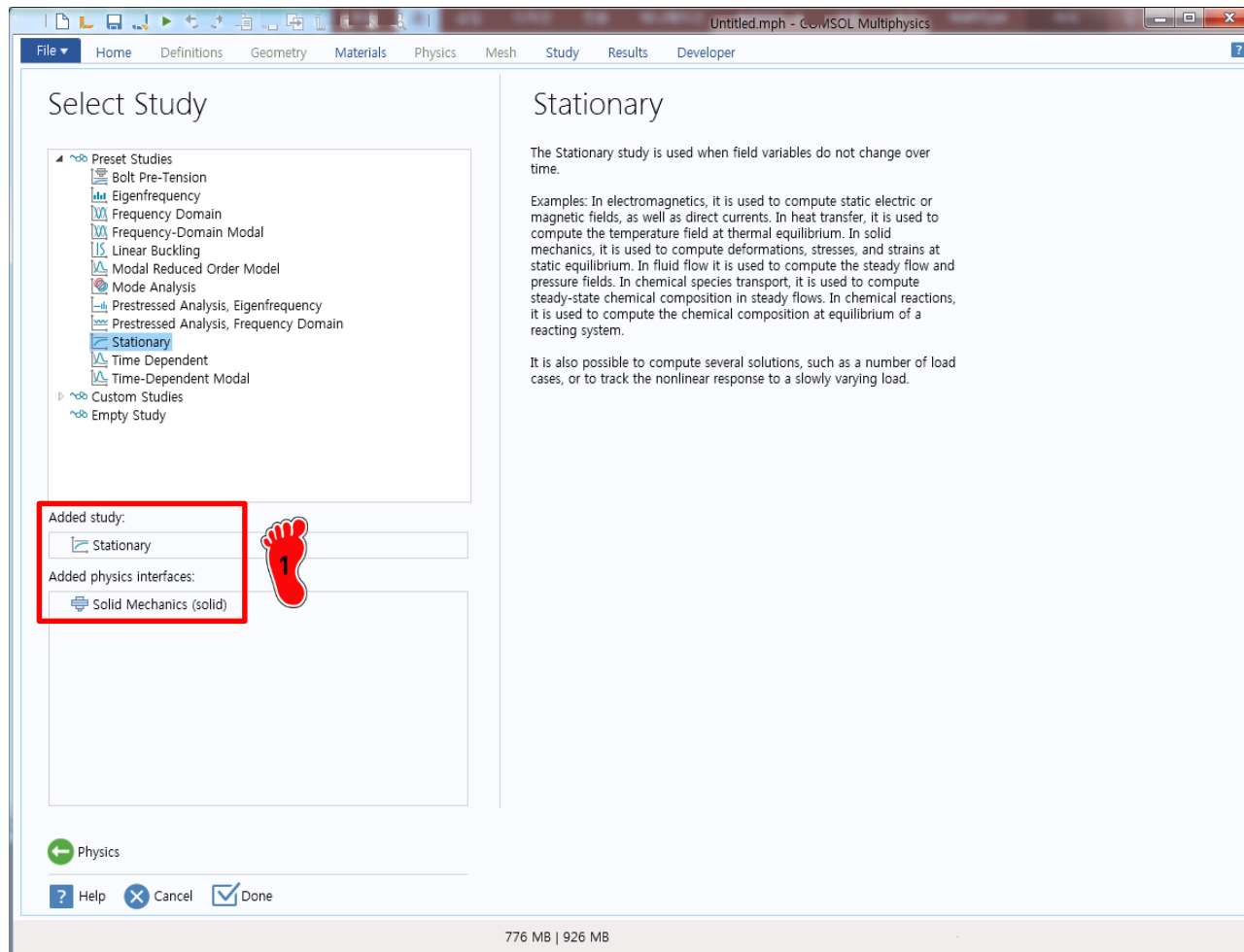
$$E_0 = 60, \nu = 0.25, h = 1$$

$$\mathbf{E}_{\text{plane stress}} = \begin{bmatrix} 64 & 16 & 0 \\ 16 & 64 & 0 \\ 0 & 0 & 24 \end{bmatrix} \quad \mathbf{E}_{\text{plane strain}} = \begin{bmatrix} 72 & 24 & 0 \\ 24 & 72 & 0 \\ 0 & 0 & 24 \end{bmatrix}$$

$$\mathbf{K}_{\text{e plane stress}} = \begin{bmatrix} 11 & 5 & -10 & -2 & -1 & -3 \\ 5 & 11 & 2 & 10 & -7 & -21 \\ -10 & 2 & 44 & -20 & -34 & 18 \\ -2 & 10 & -20 & 44 & 22 & -54 \\ -1 & -7 & -34 & 22 & 35 & 15 \\ 3 & -21 & 18 & -54 & -15 & 75 \end{bmatrix}$$

$$\mathbf{K}_{\text{e plane strain}} = \begin{bmatrix} 12 & 6 & -12 & 0 & 0 & -6 \\ 6 & 12 & 0 & 12 & -6 & -24 \\ -12 & 0 & 48 & -24 & -36 & 24 \\ 0 & 12 & -24 & 48 & 24 & -60 \\ 0 & -6 & -36 & 24 & 36 & -18 \\ -6 & -24 & 24 & -60 & -18 & 84 \end{bmatrix}$$

ENVIRONMENT SETTING



Dimension : 2D

Physics :

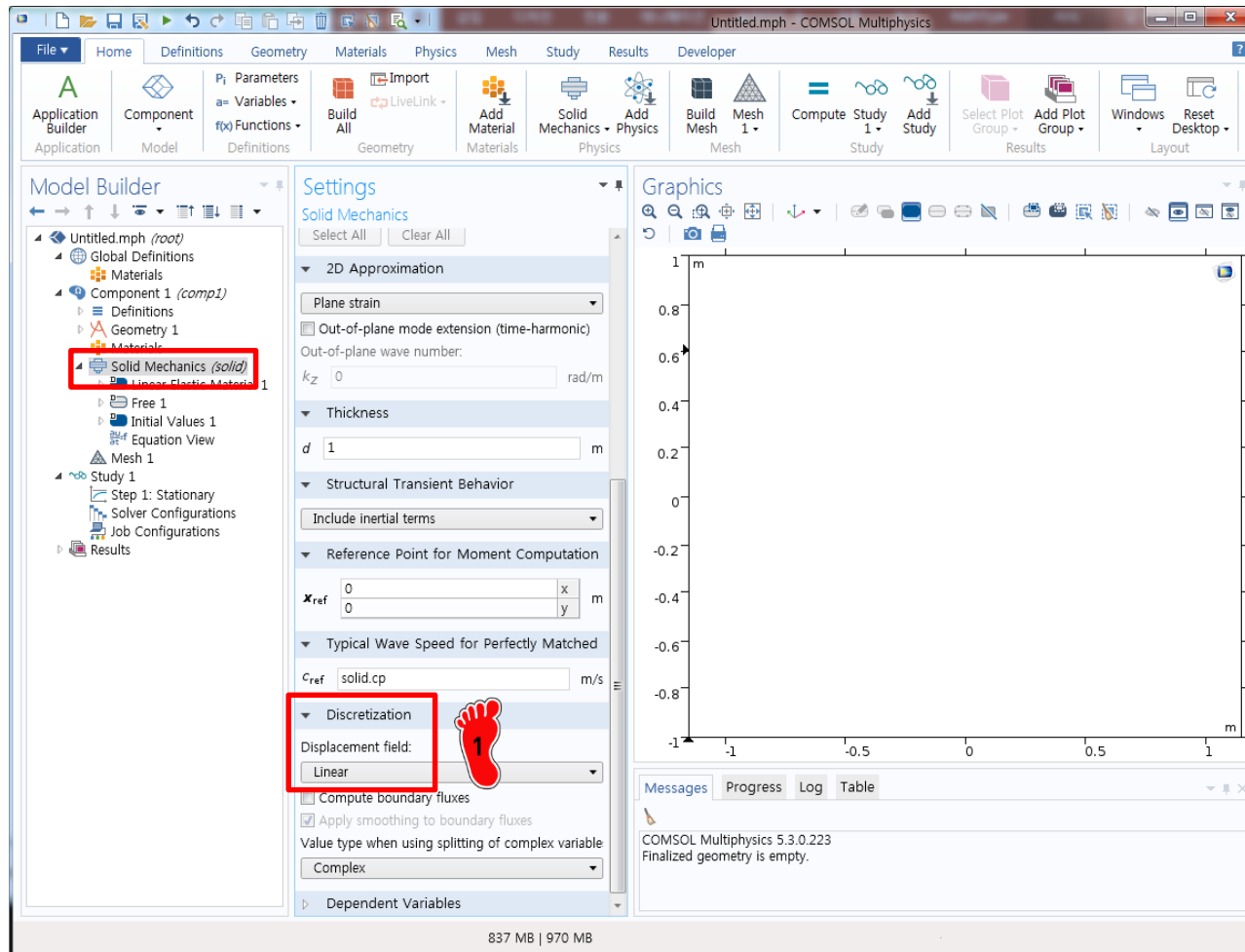
Structural Mechanics

→ Solid Mechanics

Study : Stationary

Done 클릭

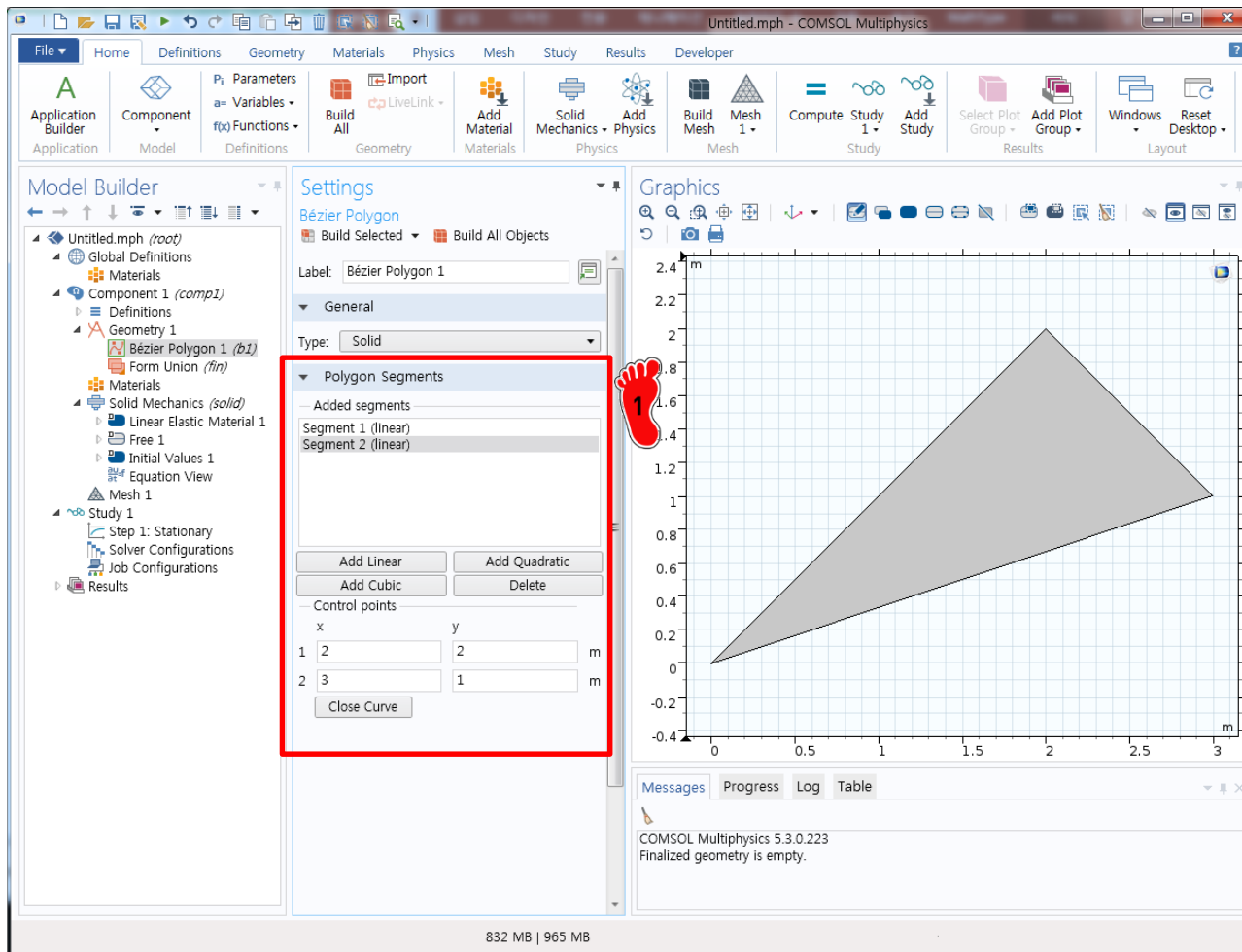
DISCRETIZATION



Solid Mechanics 클릭

Discretization 메뉴에서
Displacement field를
Linear로 변경

GEOMETRY CREATION

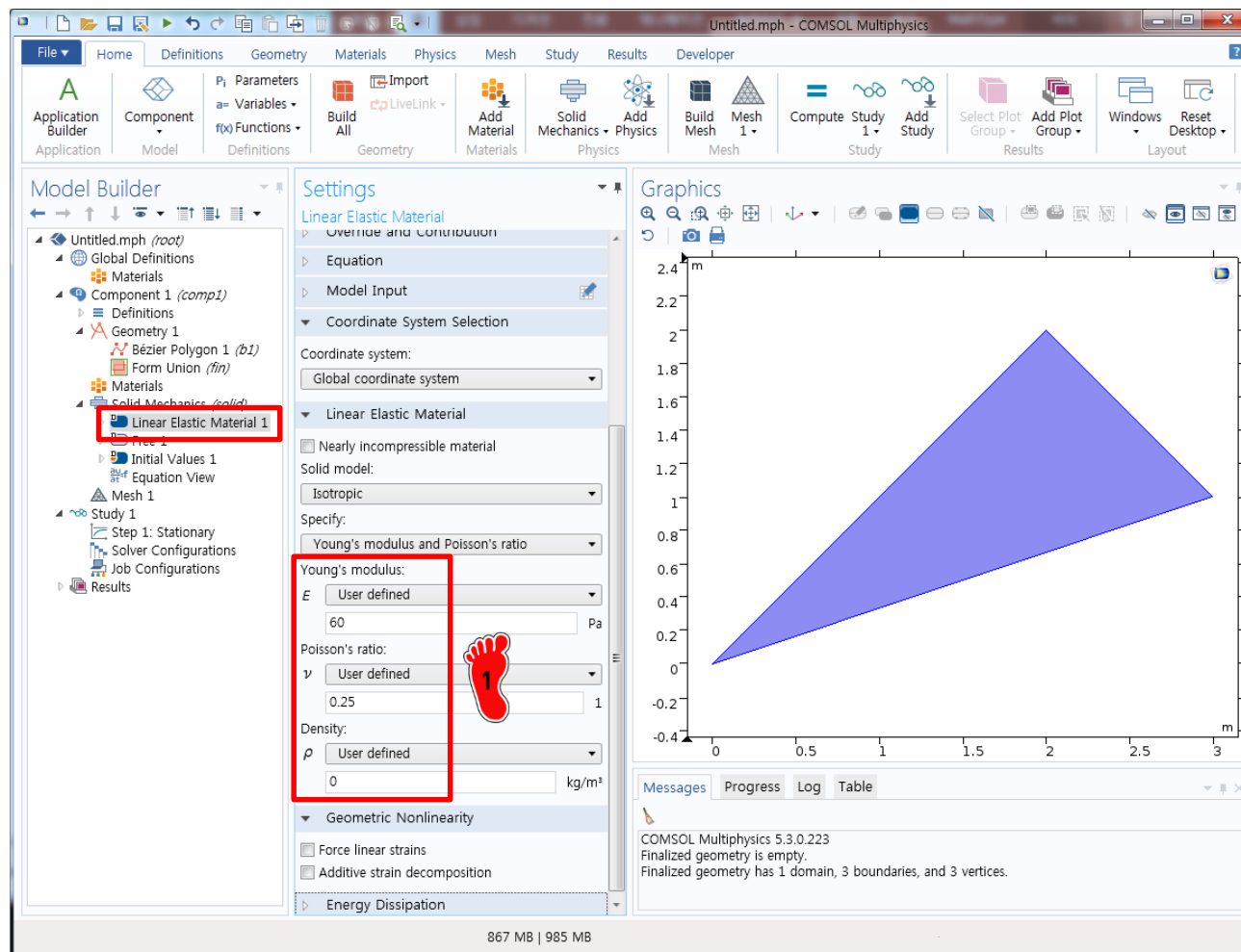


Bezier Polygon을 이용하여
삼각형 기하형상 생성

Seg. 1 : (0,0),(2,2)

Seg. 2 : (2,2),(3,1)

MATERIAL PROPERTY



Linear Elastic Material 클릭

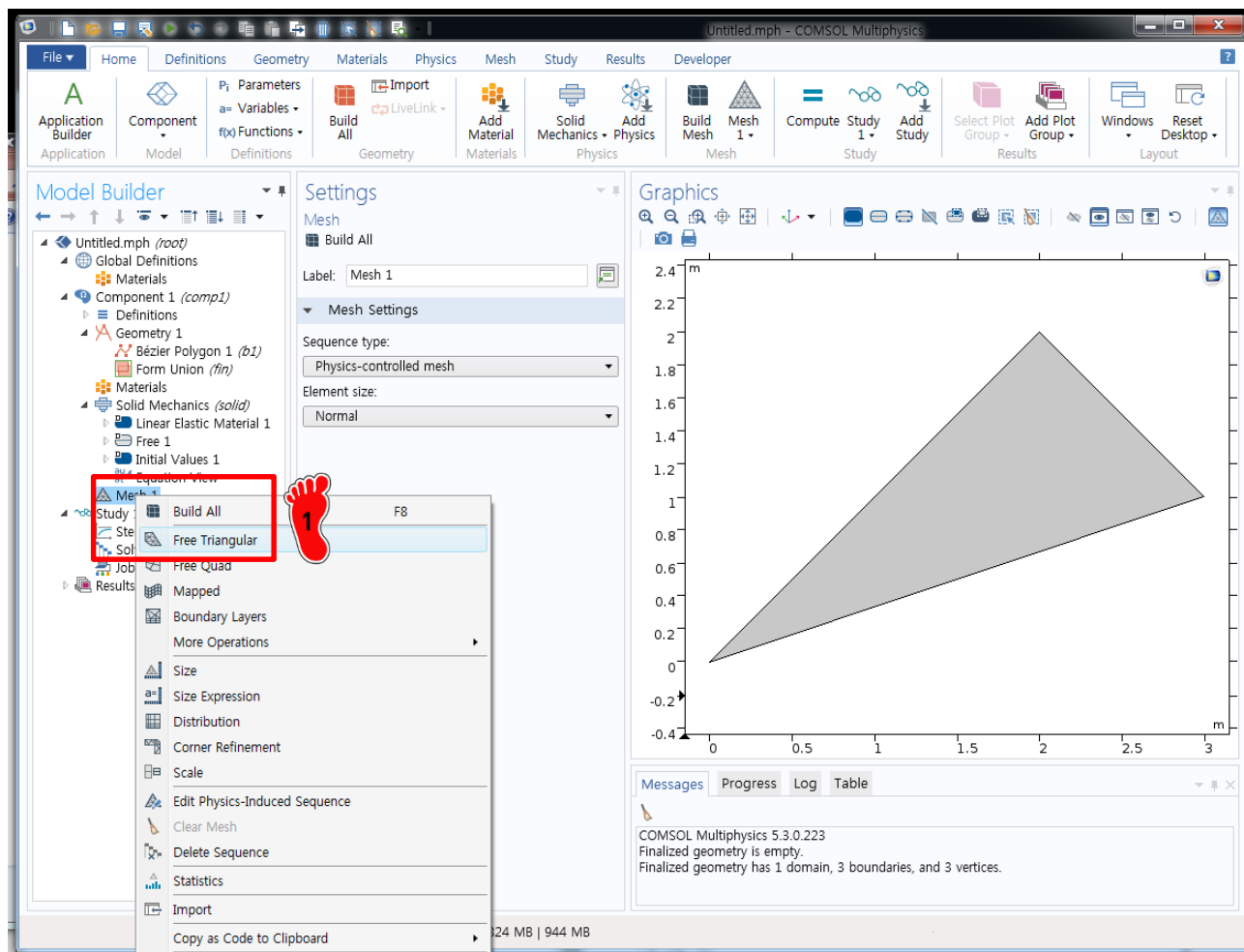
물성치 입력

E: 60

mu: 0.25

rho : 0

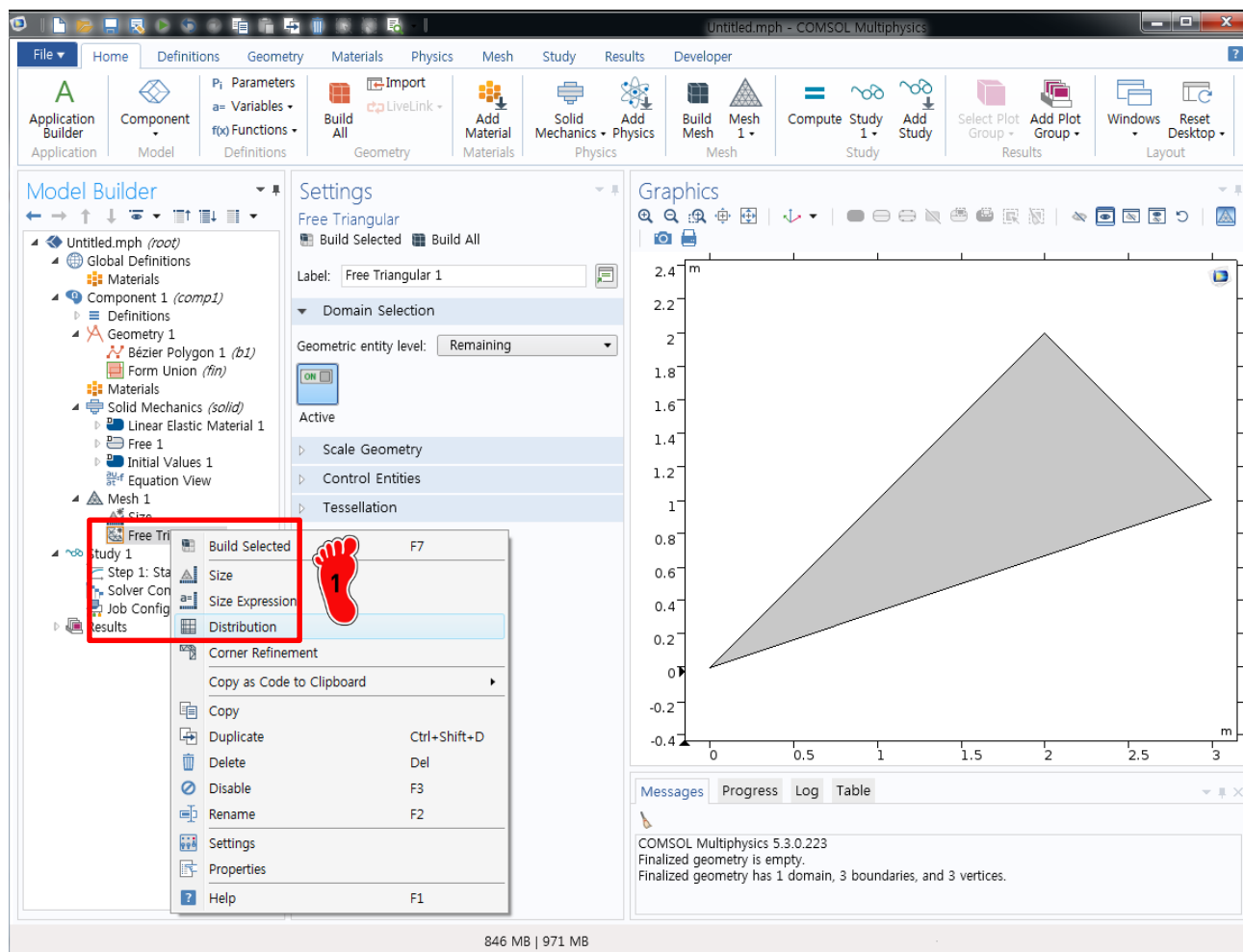
MESH



Mesh 마우스 우클릭

Free Triangular 클릭

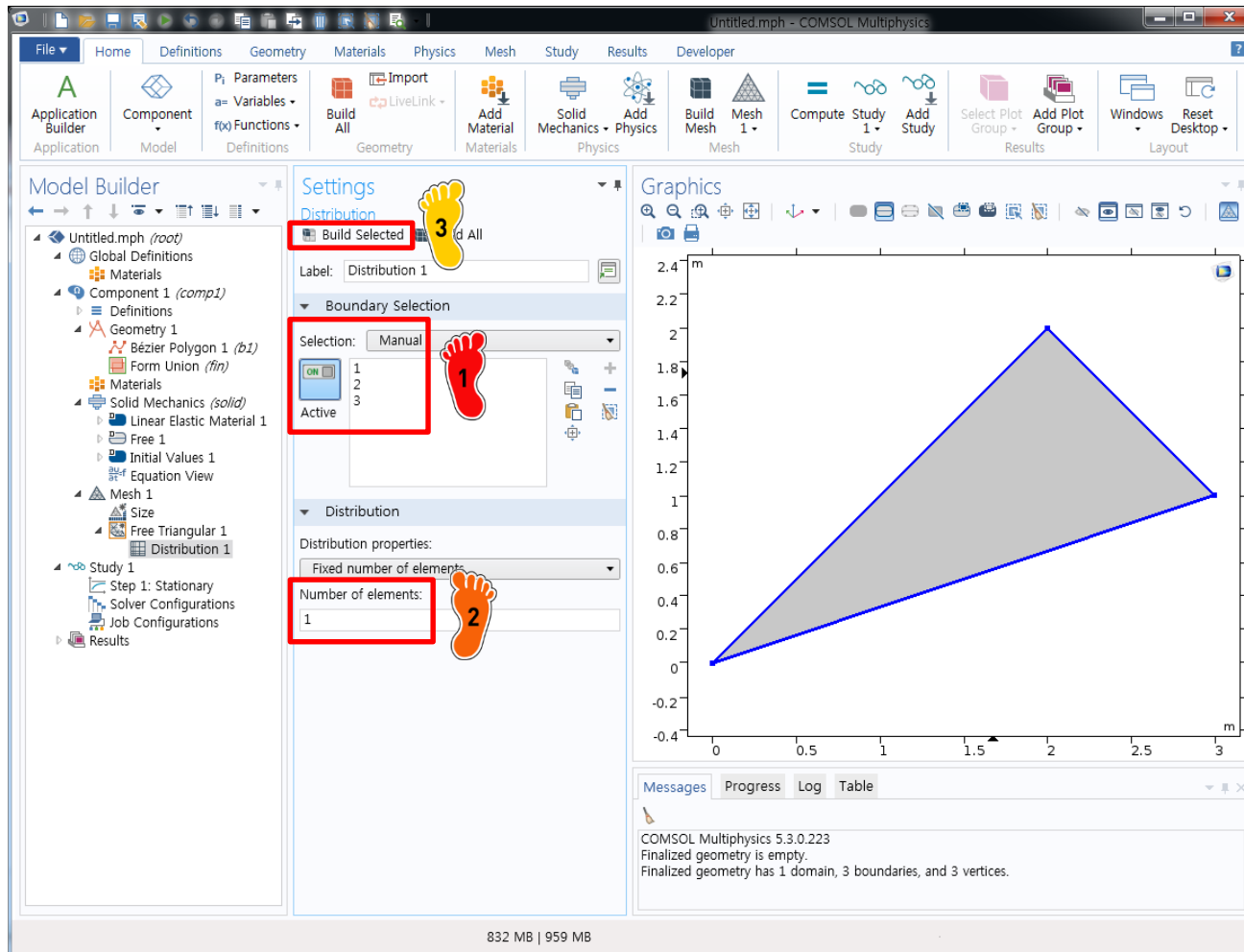
MESH



Free Triangular 마우스
우클릭

Distribution 클릭

MESH



1 3개의 boundary 선택

2 Number of elements를 1로 설정

3 1개의 삼각형 요소 생성



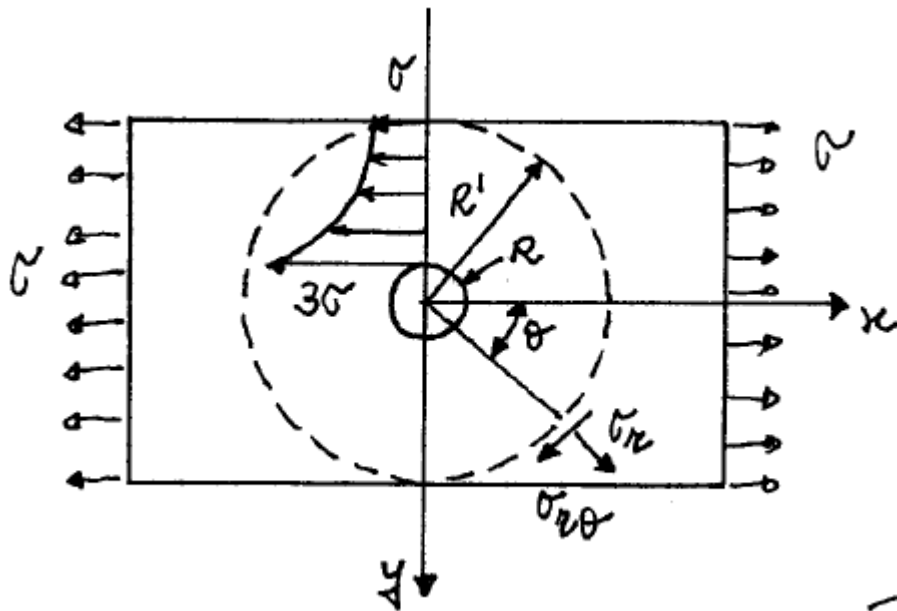
Result → Derived Values에서 System Matrix 클릭 후 Stiffness matrix 확인

$$= \begin{bmatrix} 12 & 6 & -12 & 0 & 0 & -6 \\ 6 & 12 & 0 & 12 & -6 & -24 \\ -12 & 0 & 48 & -24 & -36 & 24 \\ 0 & 12 & -24 & 48 & 24 & -60 \\ 0 & -6 & -36 & 24 & 36 & -18 \\ -6 & -24 & 24 & -60 & -18 & 84 \end{bmatrix}$$

- Stiffness matrix of Turner triangle
- 2D plane stress model
 - ✓ Kirsh's problem
- Quarter model

KIRSCH'S PROBLEM: THEORY

Infinite plate containing a circular hole (Kirsh, G. (1898), V.D.I., 42, 797-807)



Consider portion of plate within concentric circle of radius $R' \gg R$ so that stress field is not perturbed by hole (Saint-Venant's Principle)

stress field at $r = R'$ (Mohr's circle):

$$\begin{cases} \sigma_r = \frac{\sigma}{2}(1 + \cos 2\theta) \\ \sigma_{r\theta} = -\frac{\sigma}{2}\sin 2\theta \end{cases}$$

for $r = R$:

$$\begin{cases} \sigma_r = 0 \\ \sigma_\theta = \sigma(1 - 2\cos 2\theta) \\ \sigma_{r\theta} = 0 \end{cases} \rightarrow \begin{cases} \text{max: } \sigma_\theta = 3\sigma @ \theta = \frac{\pi}{2}, \frac{3\pi}{2} \\ \text{min: } \sigma_\theta = -\sigma @ \theta = 0, \pi \end{cases}$$

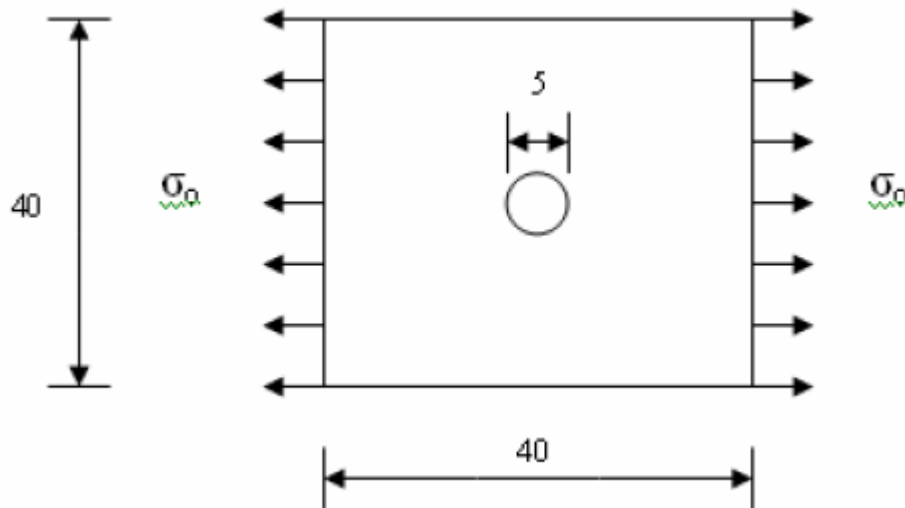
stress concentration factor = 3 independent of R

solution applicable to finite plates with width $> 4R$

solution:

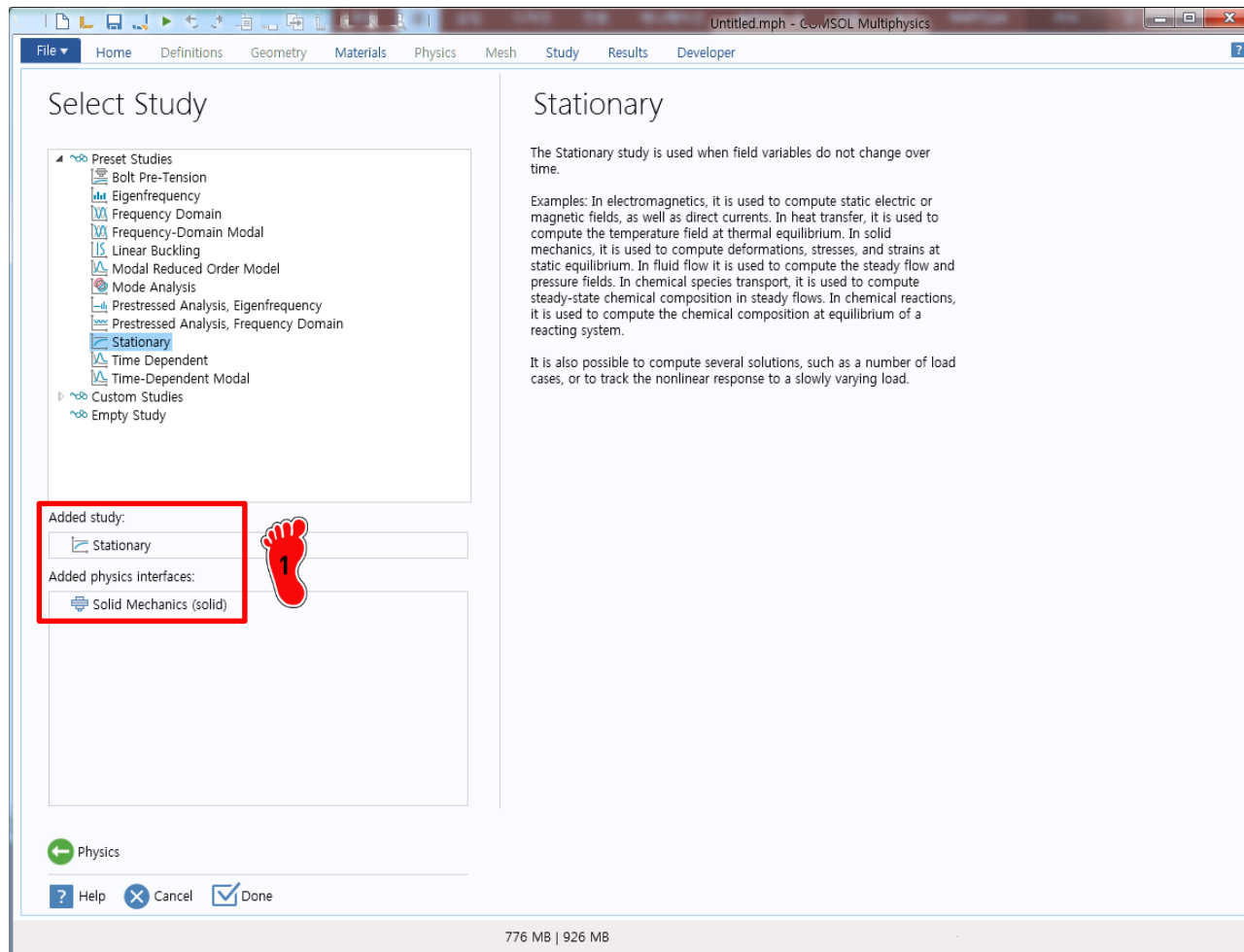
$$\begin{cases} \sigma_r = \frac{\sigma}{2}\left(1 - \frac{R^2}{r^2}\right) + \frac{\sigma}{2}\left(1 + 3\frac{R^4}{r^4} - 4\frac{R^2}{r^2}\right)\cos 2\theta \\ \sigma_\theta = \frac{\sigma}{2}\left(1 + \frac{R^2}{r^2}\right) - \frac{\sigma}{2}\left(1 + 3\frac{R^4}{r^4}\right)\cos 2\theta \\ \sigma_{r\theta} = -\frac{\sigma}{2}\left(1 - 3\frac{R^4}{r^4} + 2\frac{R^2}{r^2}\right)\sin 2\theta \end{cases}$$

KIRSCH'S PROBLEM: FEM



- 2D approximation
 - Plane stress
 - Plane strain
- Material Properties
 - $E = 200 \times 10^9$
 - $\nu = 0.3$
- Element Properties
 - Thickness = 1?
- Loads: $\sigma_0 = 1$
- BCs: none

ENVIRONMENT SETTING



Dimension : 2D

Physics :

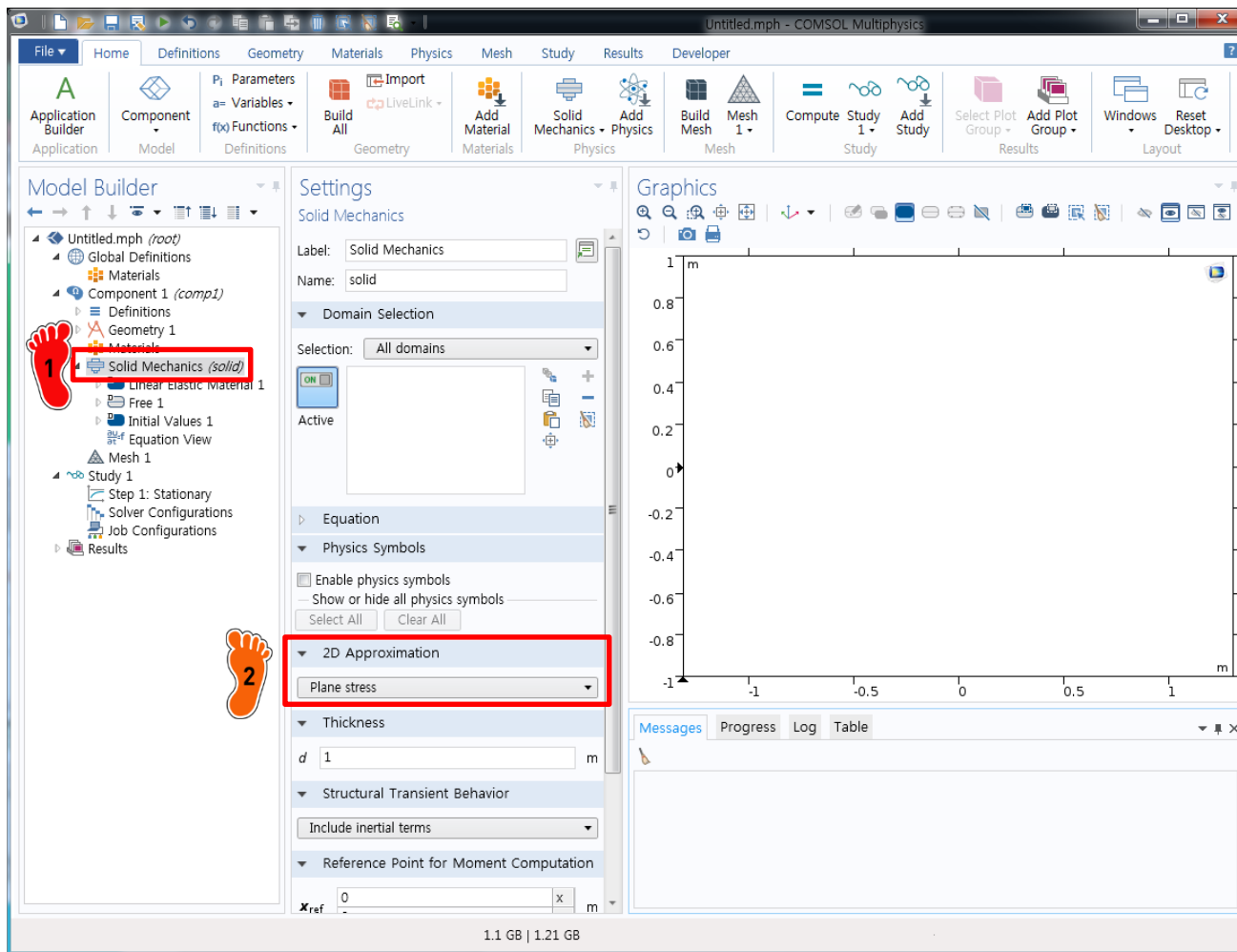
Structural Mechanics

→ Solid Mechanics

Study : Stationary

Done 클릭

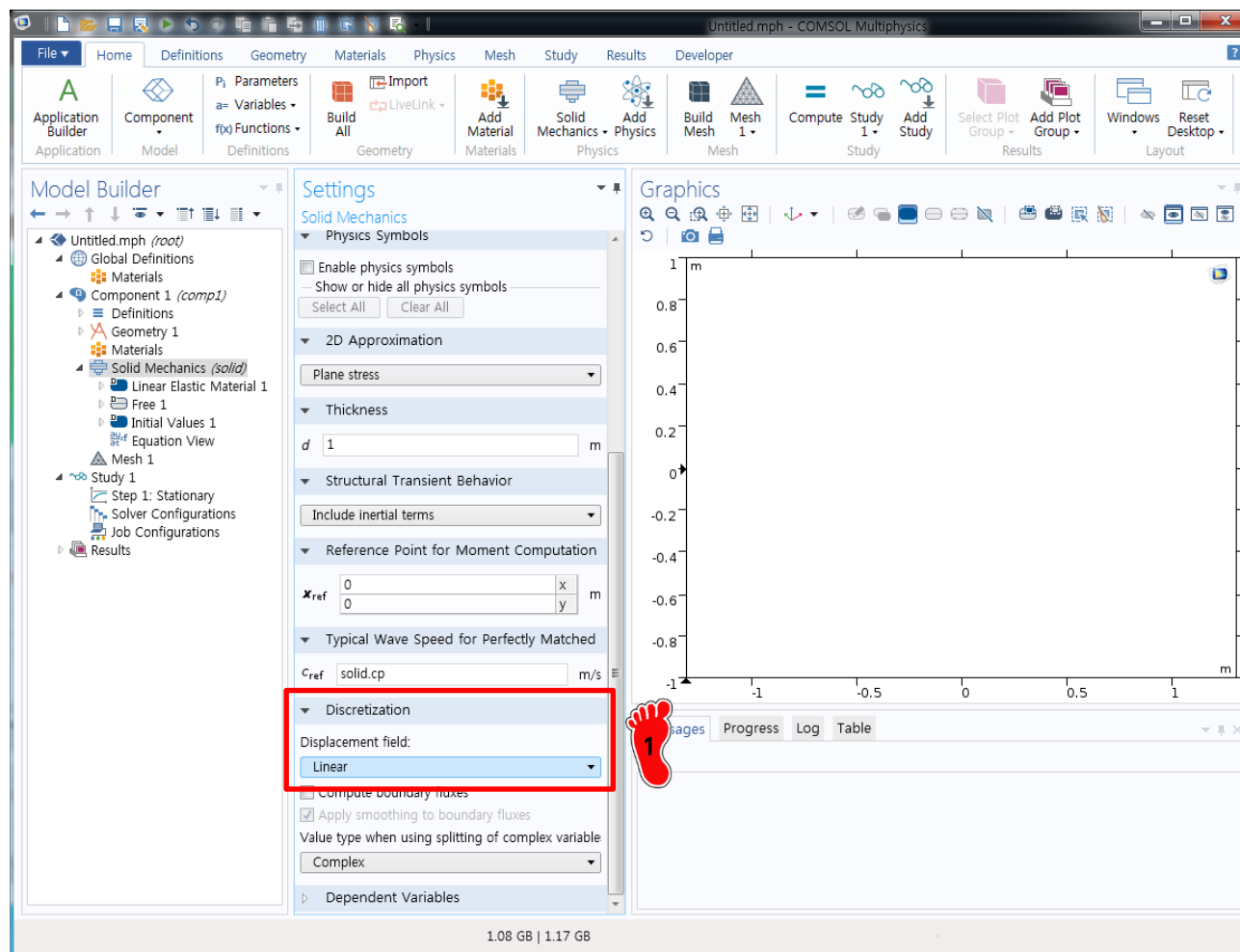
PLANE STRESS



1 Solid Mechanics 클릭

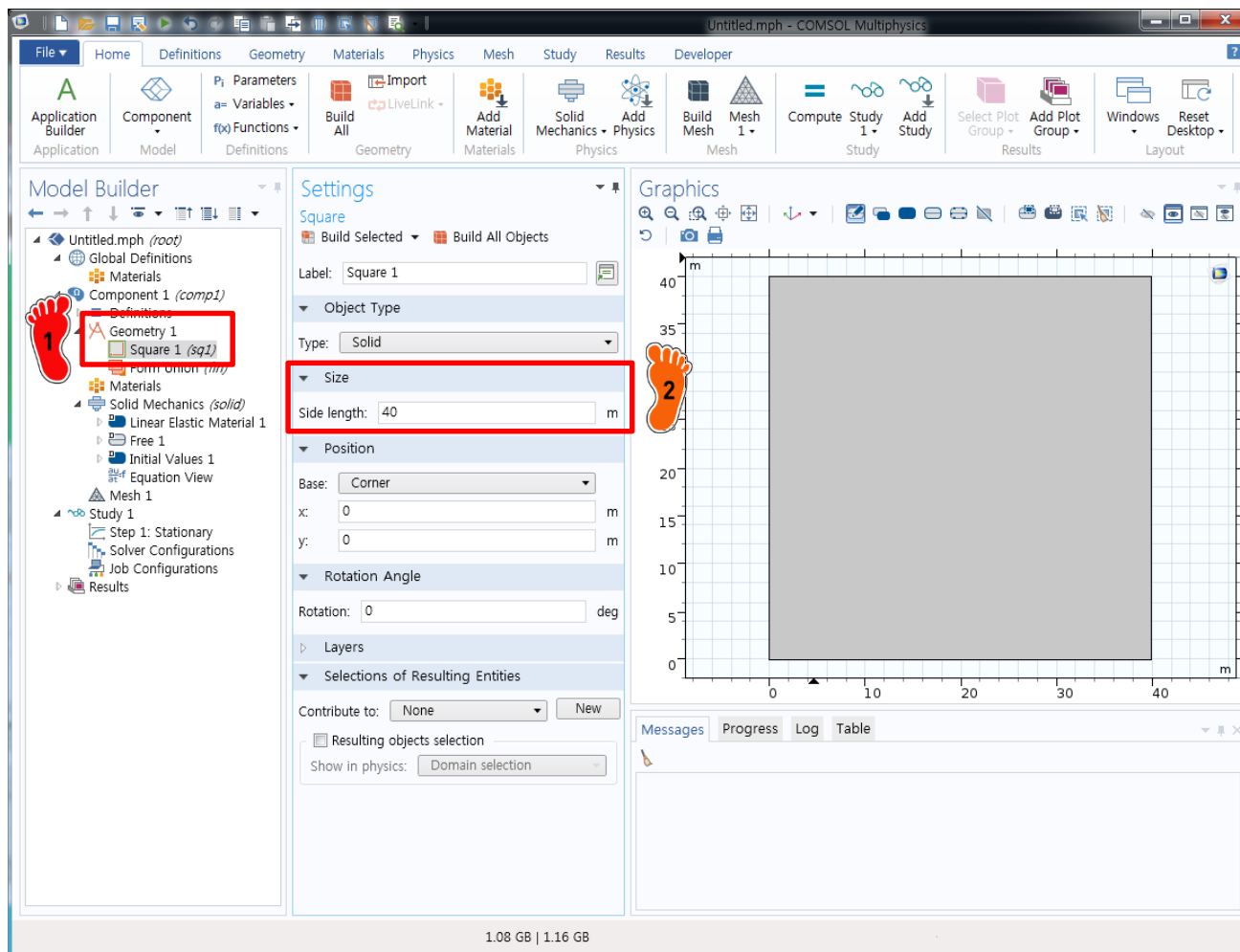
2 2D Approximation
Plane stress 로 변경

DISCRETIZATION



Discretization
Displacement field를
Linear로 변경

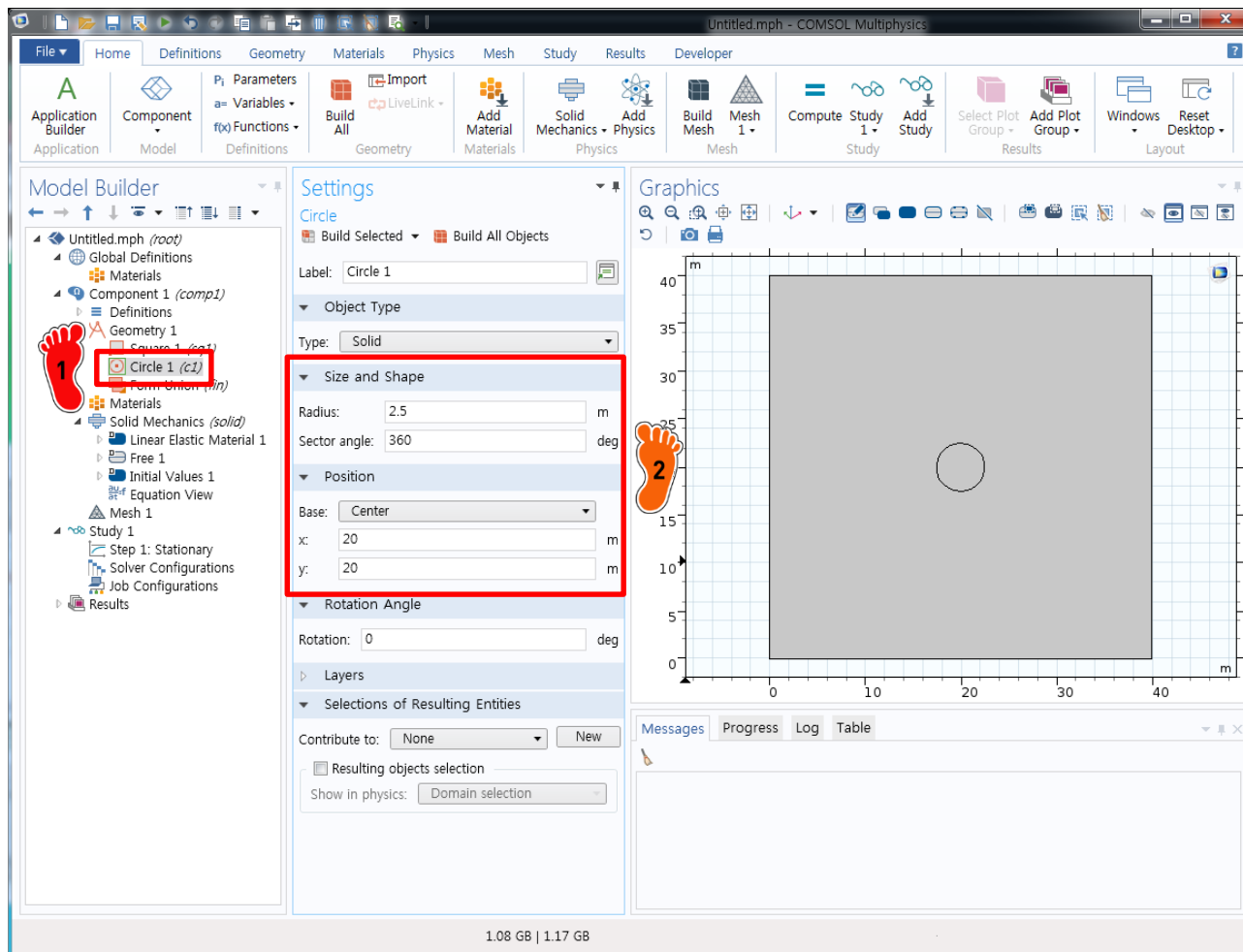
GEOMETRY CREATION



1 Square 생성

2 길이 40의 정사각형 생성

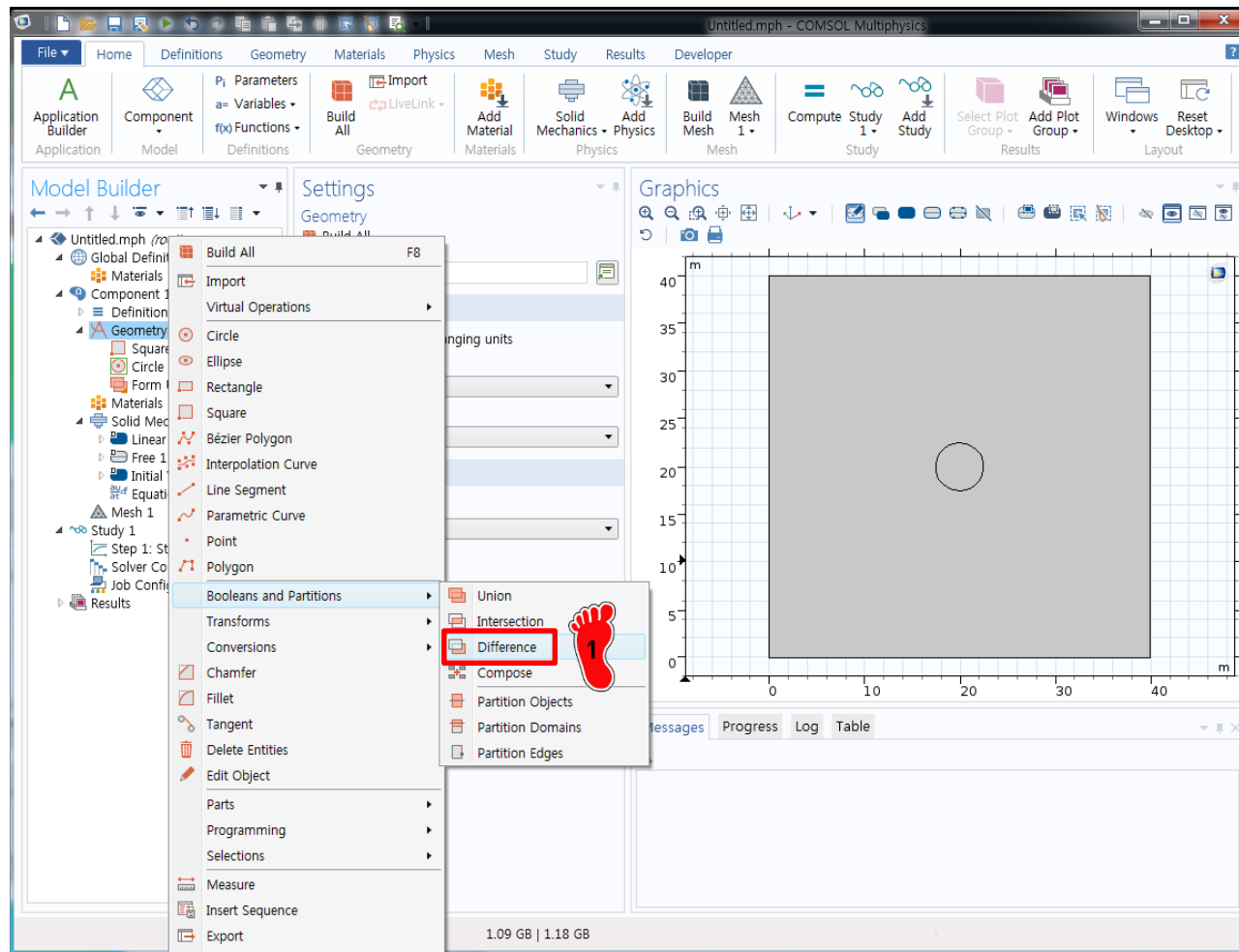
GEOMETRY CREATION



1 Circle 생성

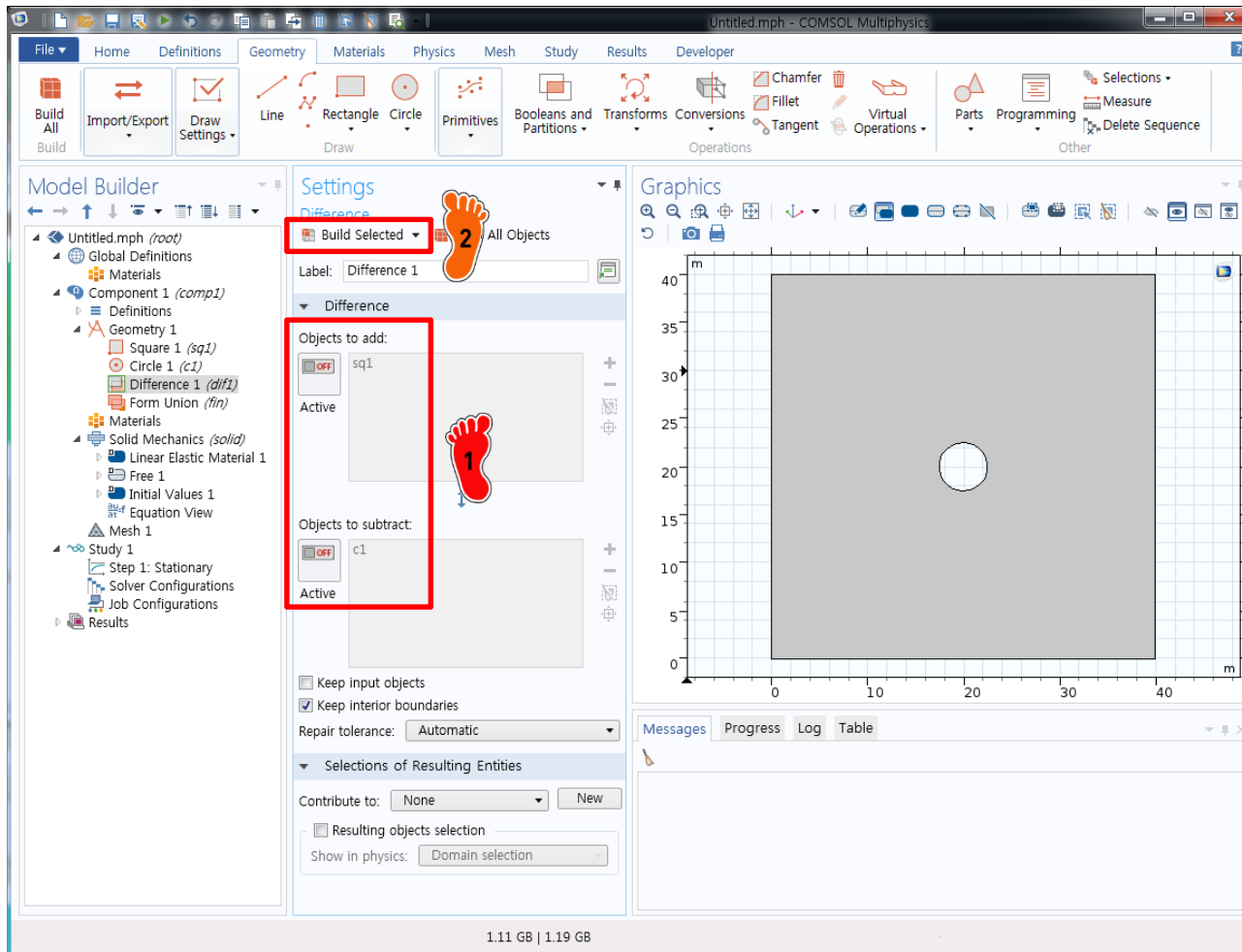
2 중심 (20,20)
반지름 2.5 원 생성

GEOMETRY CREATION



Geometry 오른쪽 클릭
→ Booleans and Partitions
→ Difference 클릭

GEOMETRY CREATION

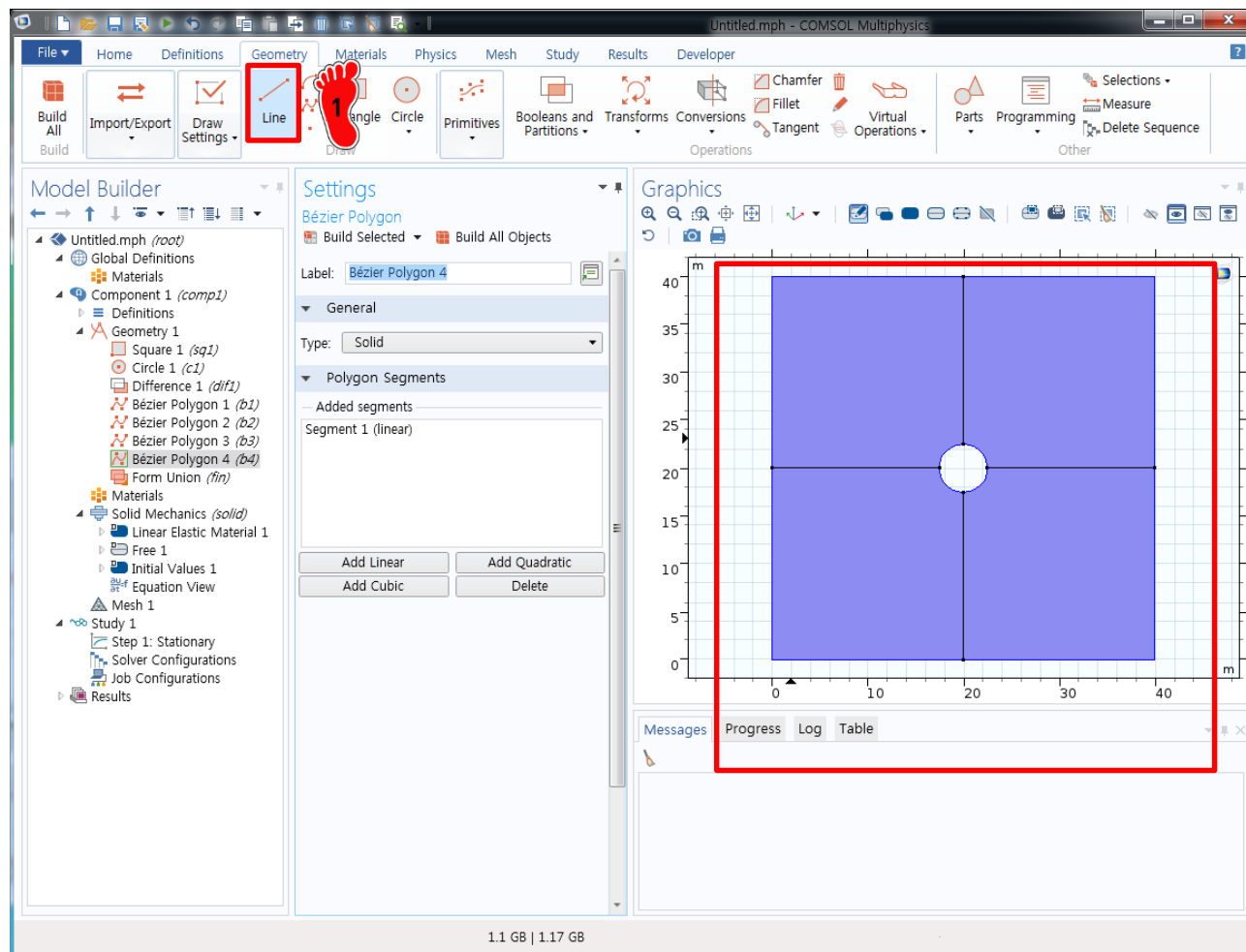


1 Objects to add
사각형 선택

Objects to subtract
원 선택(Active ON)

2 Build Selected 클릭

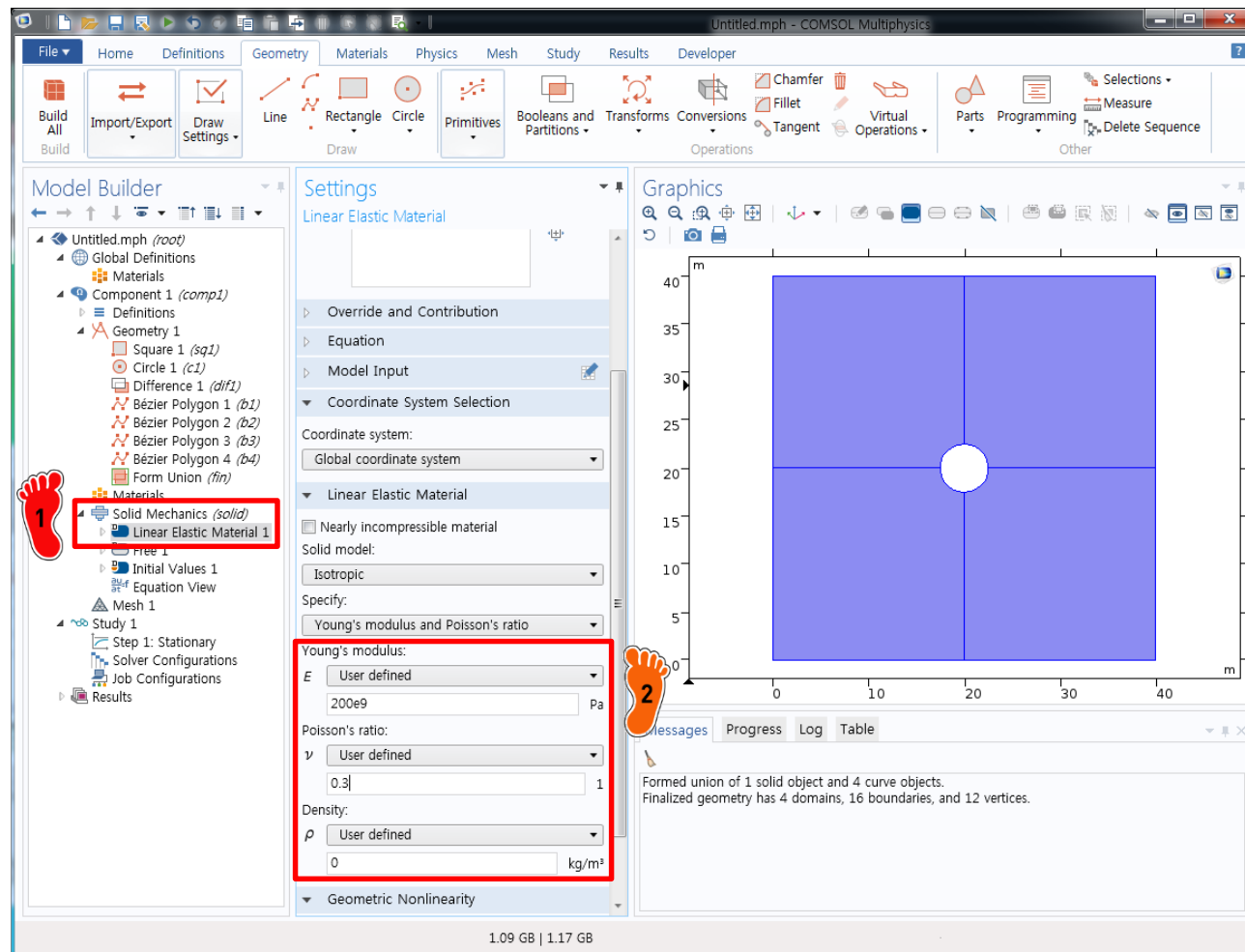
GEOMETRY CREATION



위쪽 Geometry tab에서
line 메뉴를 이용

왼쪽 마우스 클릭으로 선분
4개 생성(오른쪽 클릭: 완료)

MATERIAL PROPERTY

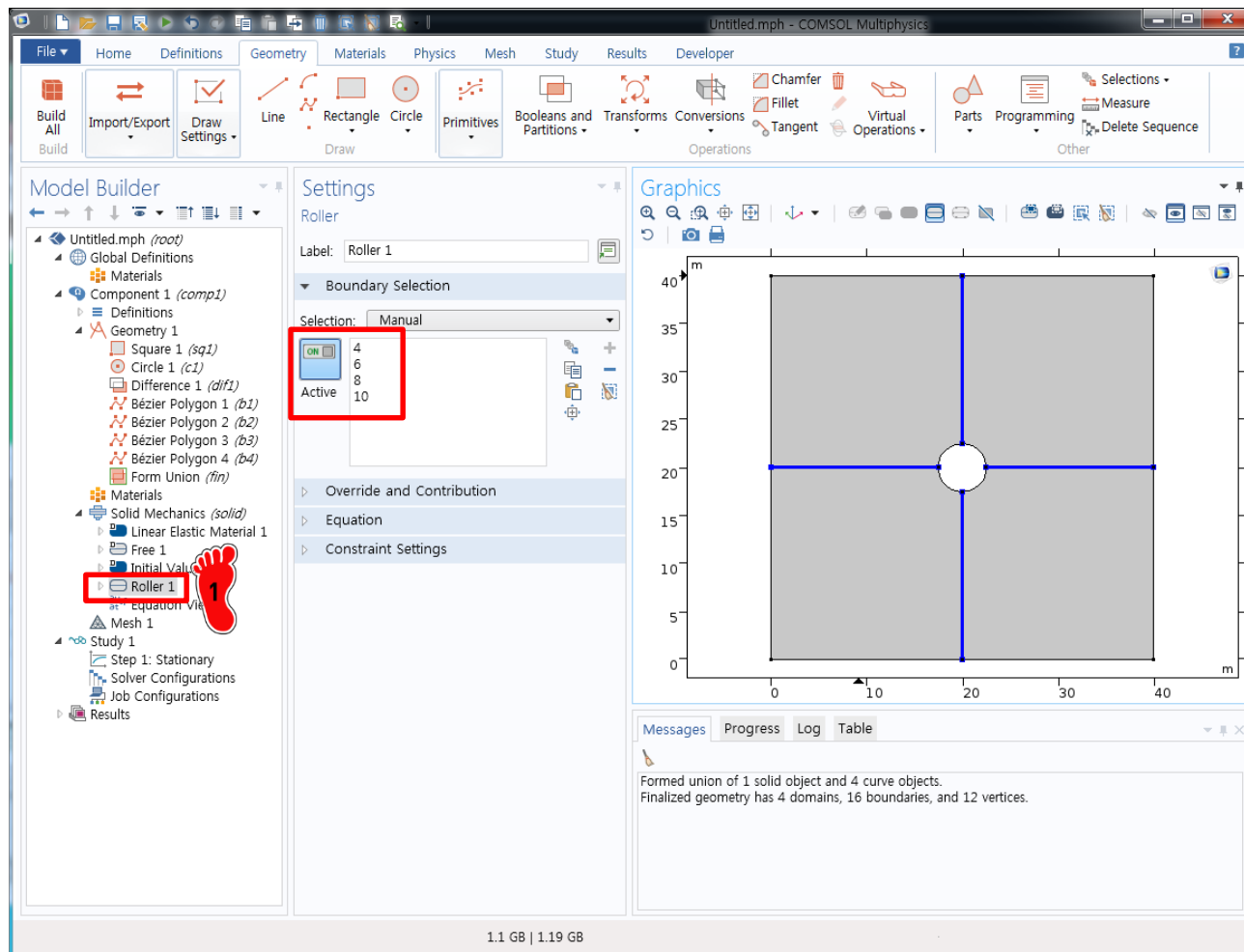


Linear Elastic Material 클릭

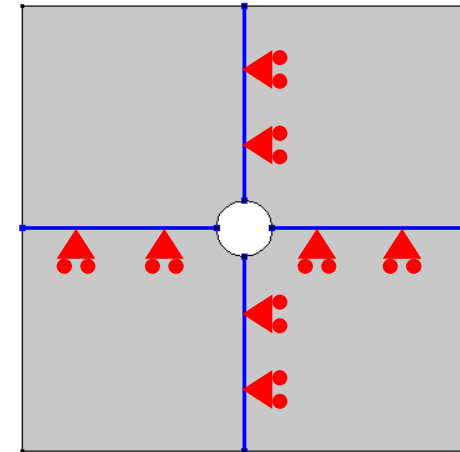


E: 200e9
mu: 0.3
rho : 0

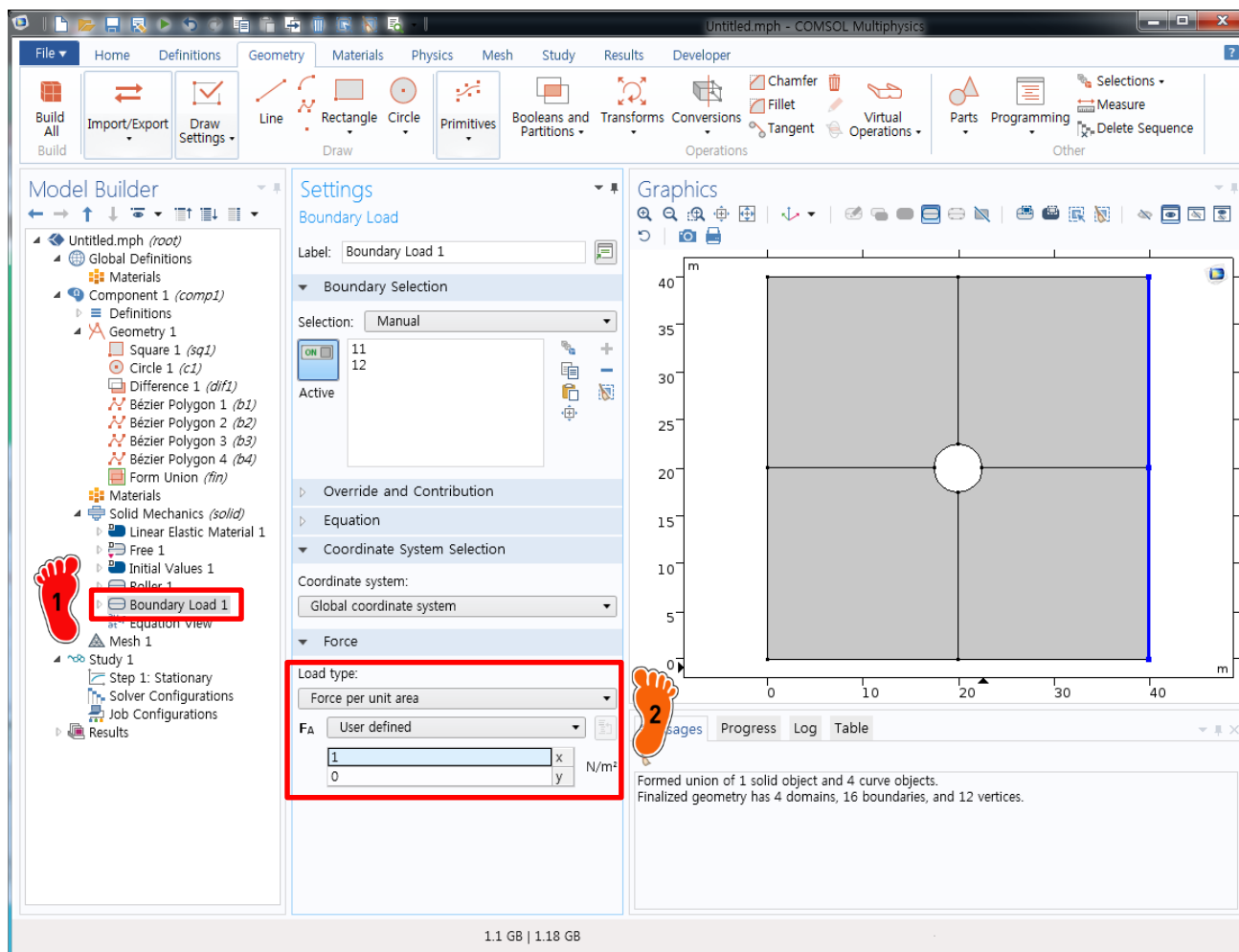
BOUNDARY CONDITION



사각형 내부 4개 변을
Roller 조건으로 입력

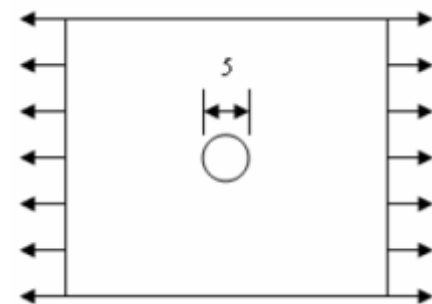


LOADING CONDITION

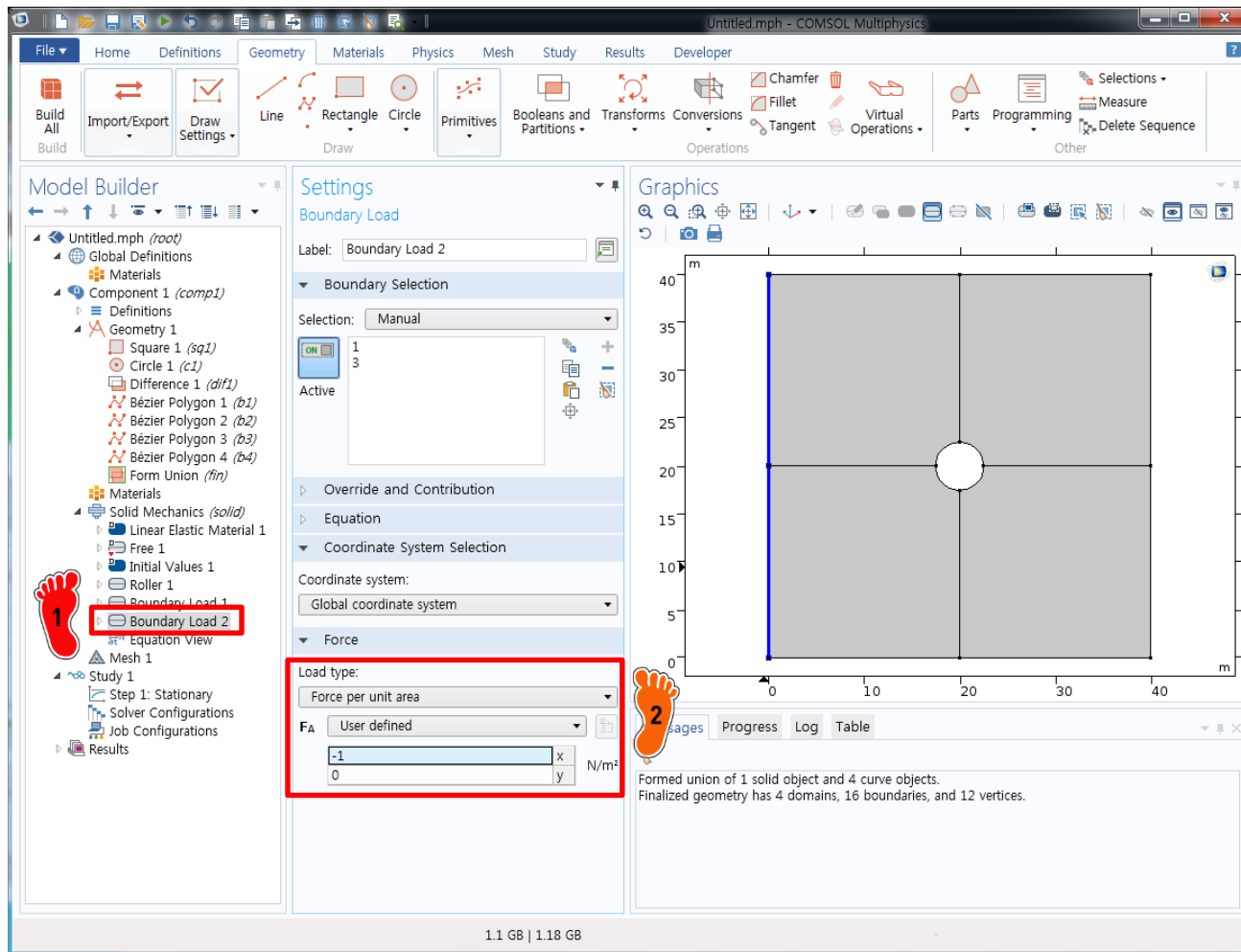


1 Boundary Load 생성

2 오른쪽 두 변에 x 방향
1 입력 (단위는 N/m²임)

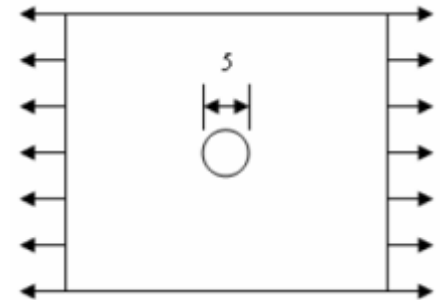


LOADING CONDITION



1 Boundary Load 생성

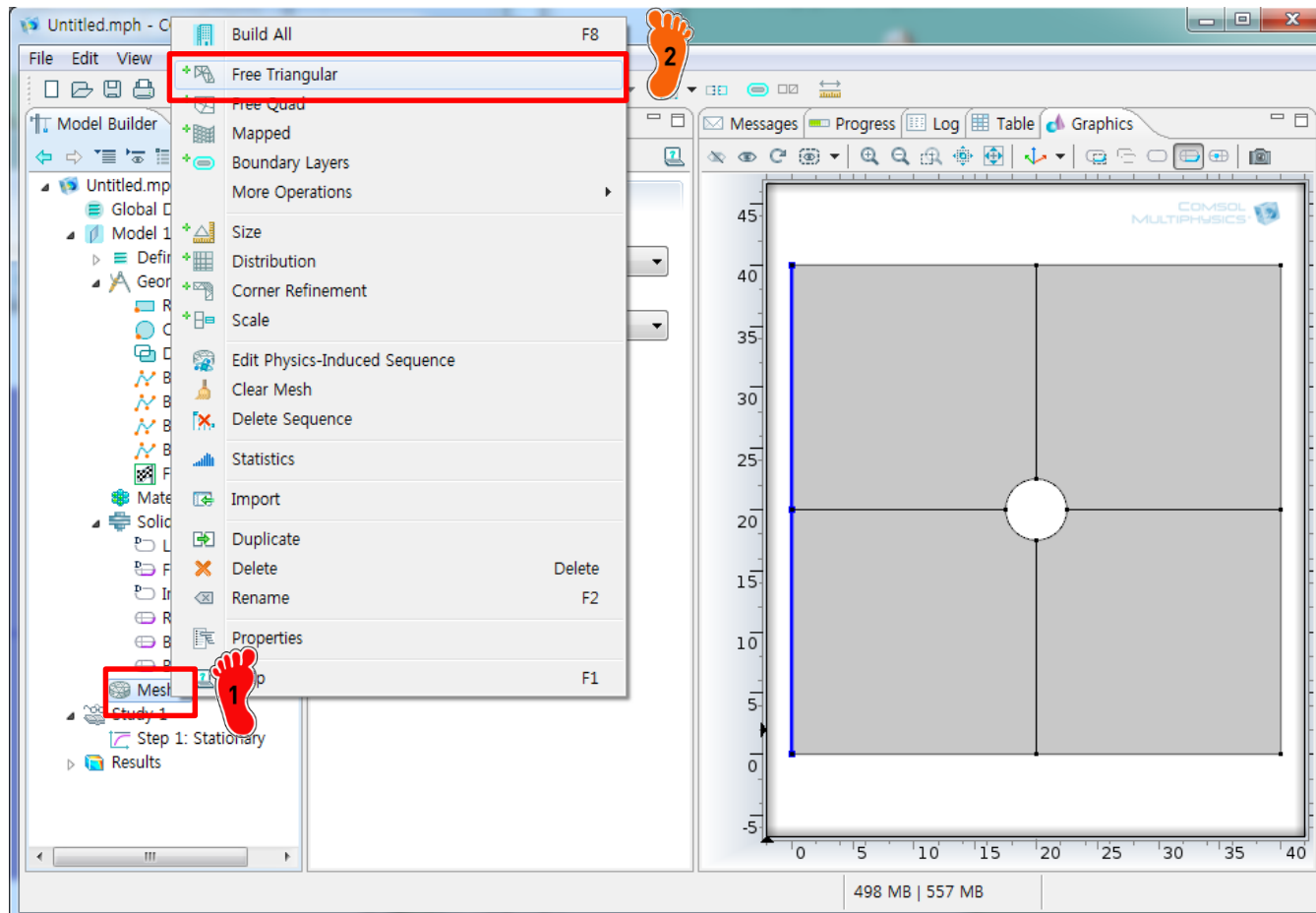
2 오른쪽 두 변에 x 방향
-1 입력 (단위는 N/m²임)



MESH

1 Mesh 마우스 우클릭

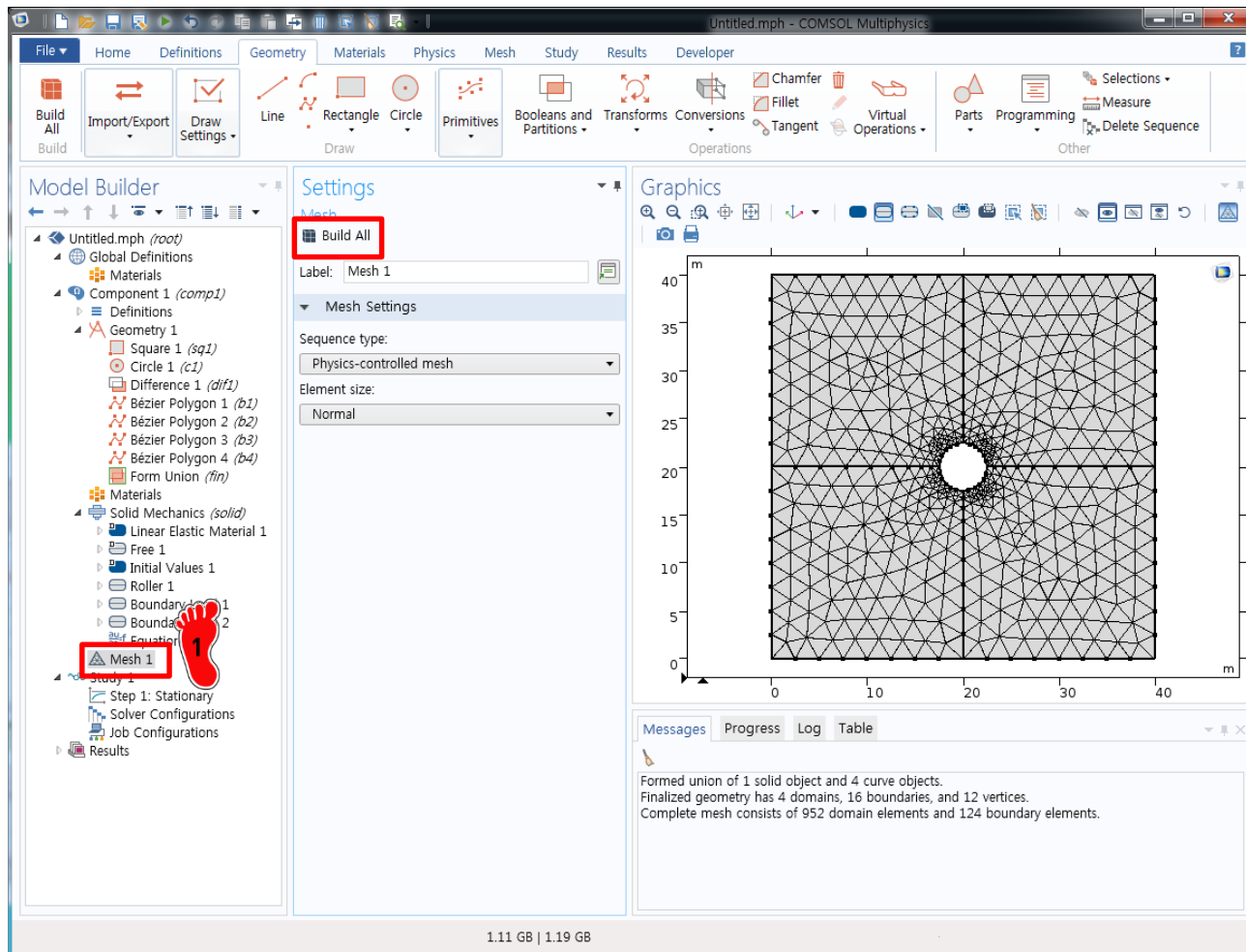
2 Free Triangular 클릭



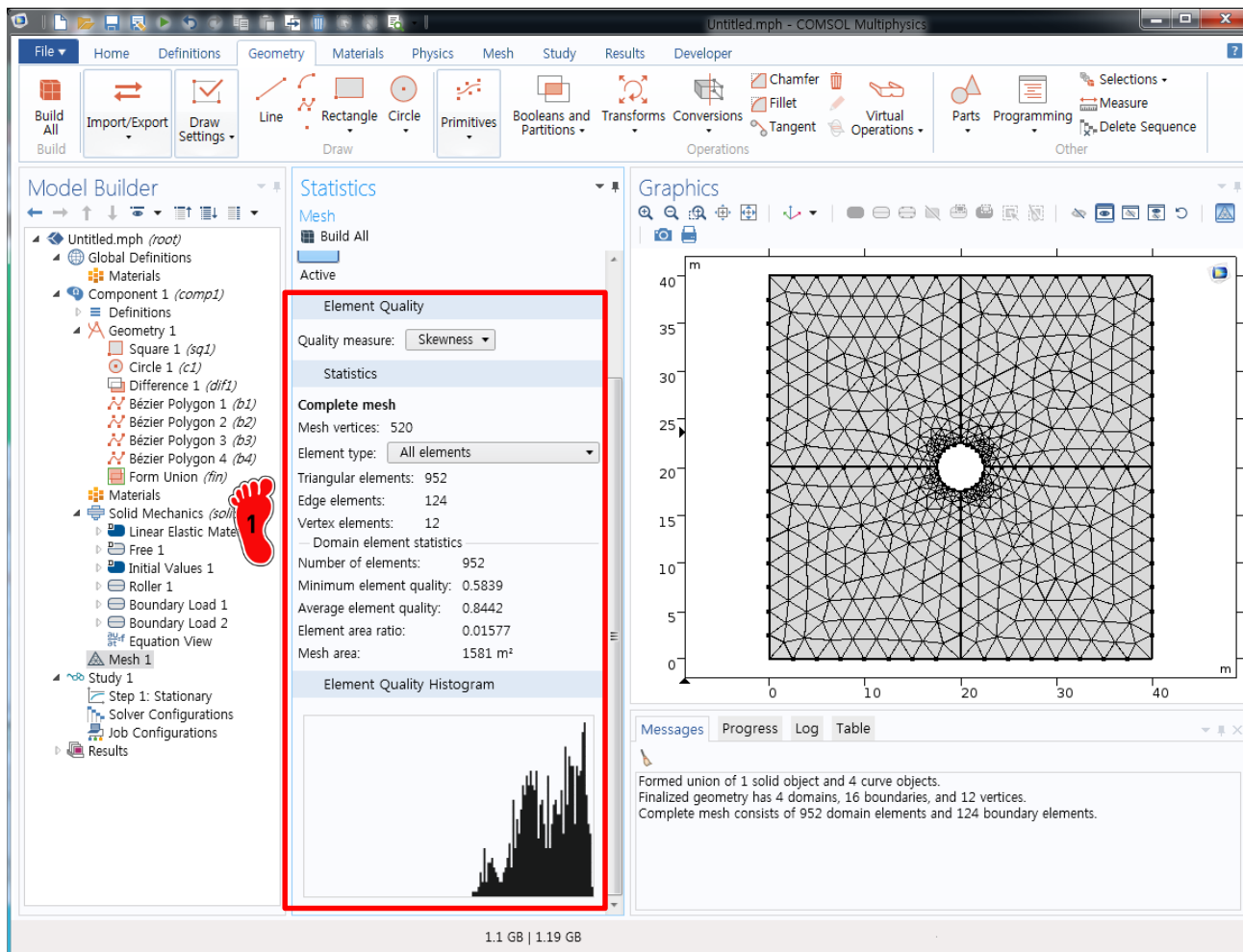
MESH



기본 Mesh 생성(삼각형)



MESH



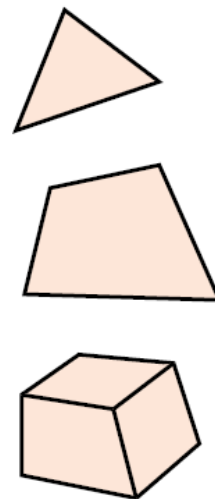
Mesh 우클릭
→ Statistics 클릭

요소 개수, DOF, Quality
분포 정보 등을 보여줌

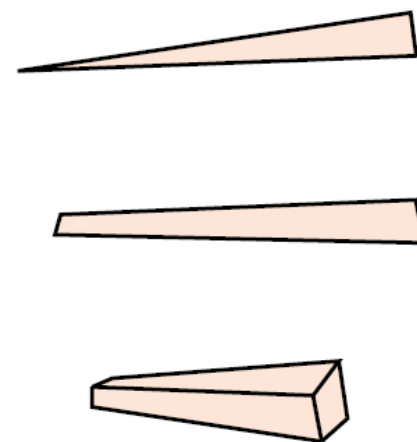
2D/3D BAD ASPECT RATIO ELEMENT

- “thin” structures modeled as continuous bodies
 - Elongated or “skinny” element
- Aspect ratio
 - Ratio between its largest and smallest dimension
 - > 3 : caution
 - > 10 : alarm

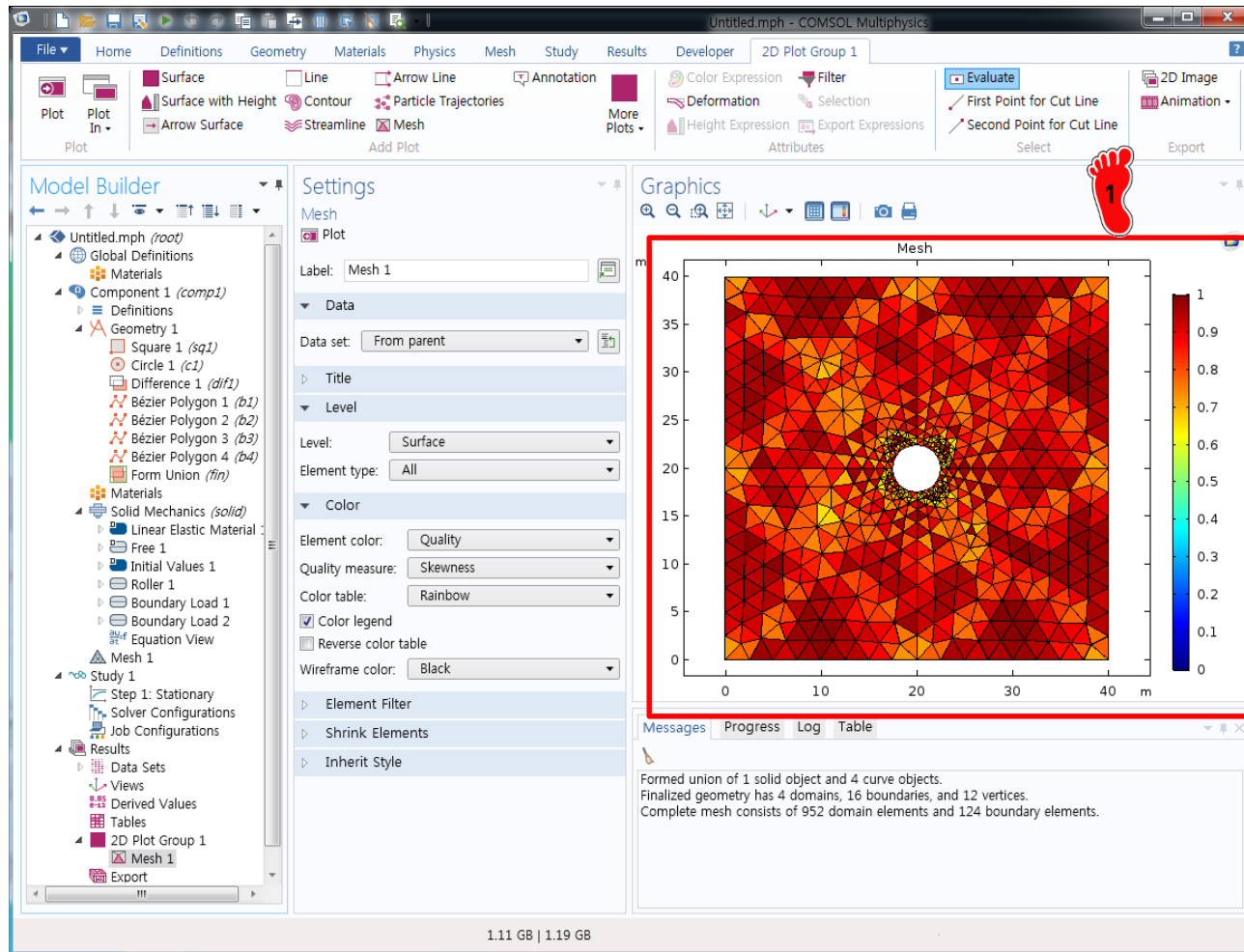
Good



Bad



MESH

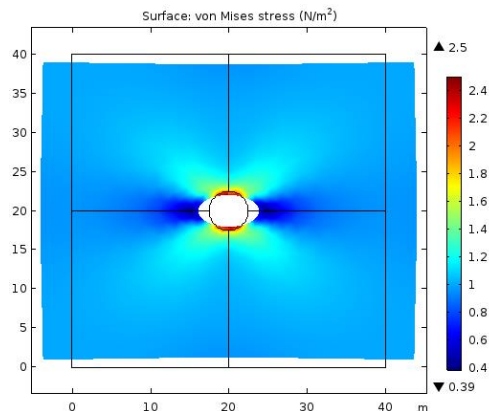


Mesh 우클릭 → Plot

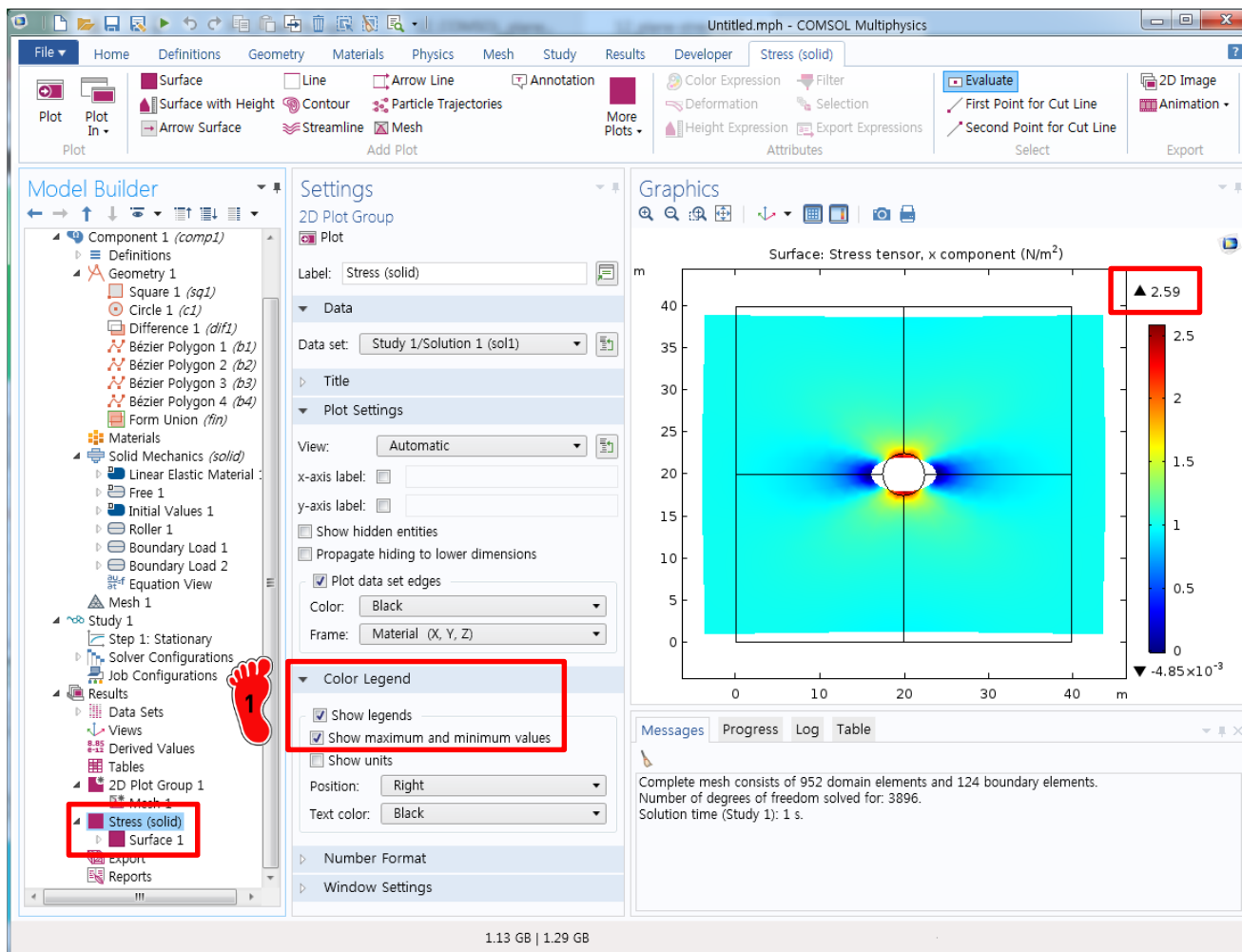
빨간색에 가까운 요소는
정삼각형에 가까운 요소
(Quality가 1에 가까움)

파란색에 가까운 요소는
정삼각형과 먼 요소
(Quality가 1에서 멀어짐)

compute를 클릭하여
해석 수행



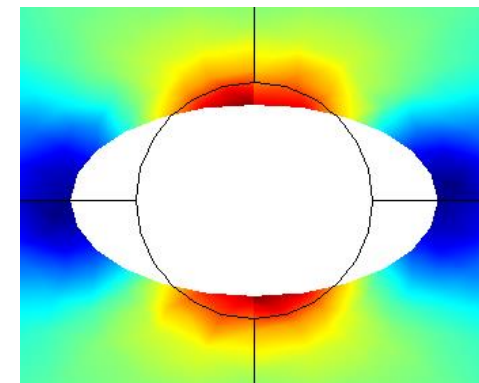
POST-PROCESSING



1 Results → Stress(solid) 클릭
Color Legend에서 Show maximum and minimum values 체크

2 Surface 클릭후 Expression을 "solid.sx"(normal stress)로 변경 후 plot

최대 수직 응력은 원에 접하는 부분에서 약 2.59 Pa



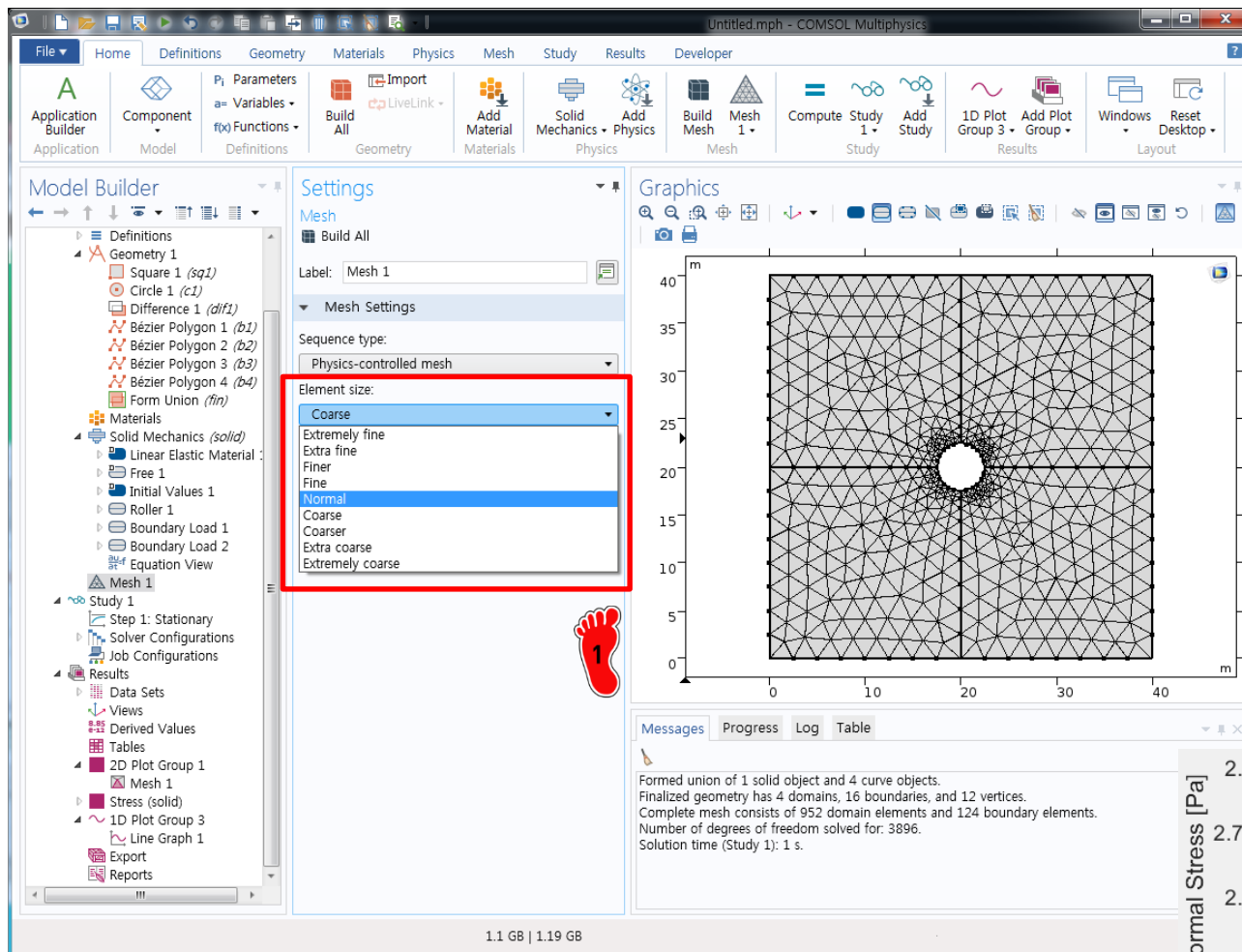
Expression

Expression:

solid.sx



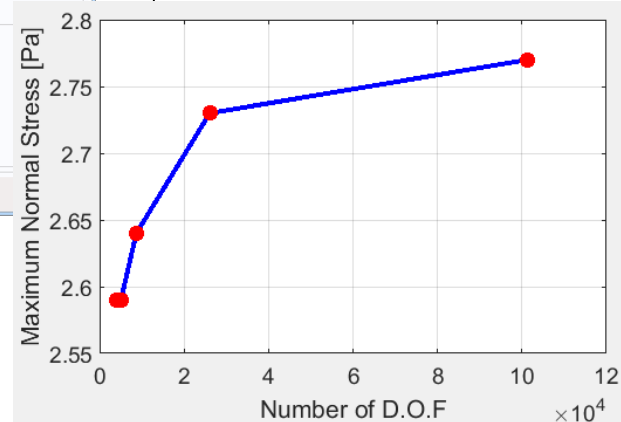
MESH REFINEMENT



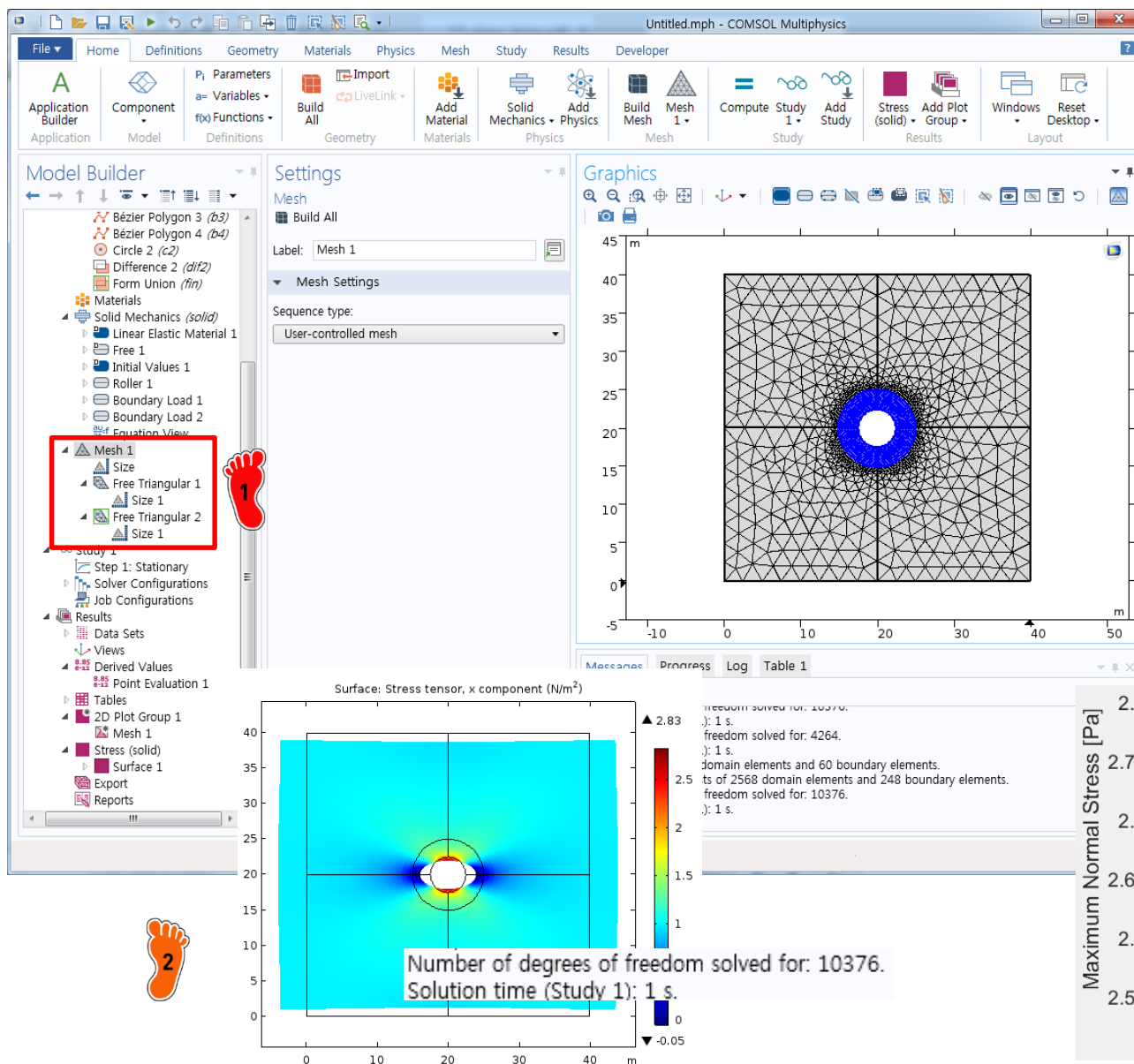
1 Mesh에서 Element Size
클릭 → Fine으로 변경 후
Build All → Compute

2 위와 같은 방식으로 요소 수
를 증가시켜 응력 결과값을
확인 출력한 그래프

Analytical solution 인
 $3 \times \sigma$ 대비 작은
값으로 수렴해 감

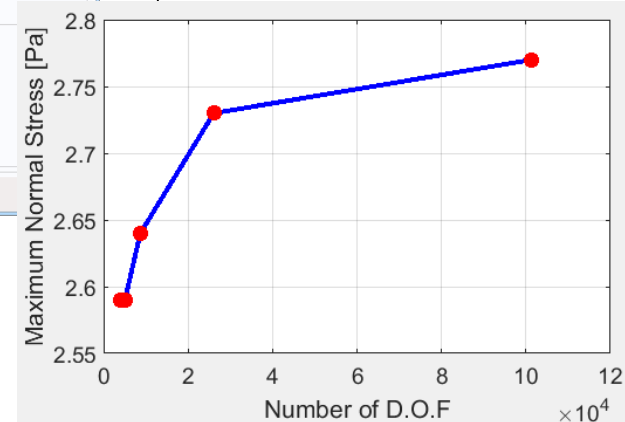


MESH REFINEMENT



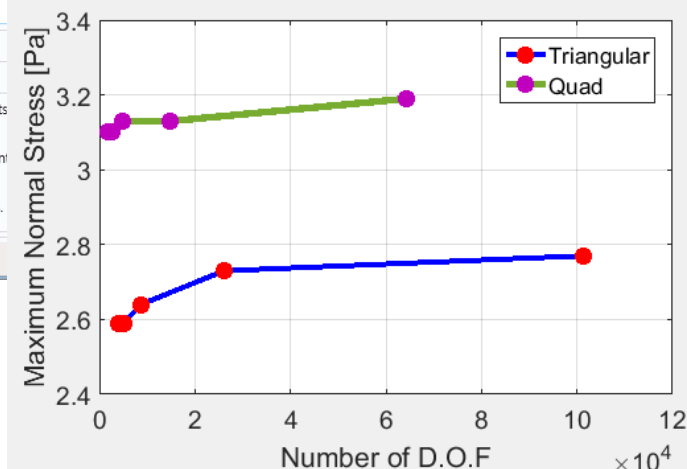
1 Geometry를 분할하여 응력이 높게 나타나는 곳만 fine mesh 적용

2 자유도 수 약 10,000개 정도로 더 정확한 결과 계산





삼각형보다 사각형 메시가
DOF수에 따라 변화 적음



$$\frac{r}{d} = \frac{2.5}{35} = 0.07, K \approx 2.8$$

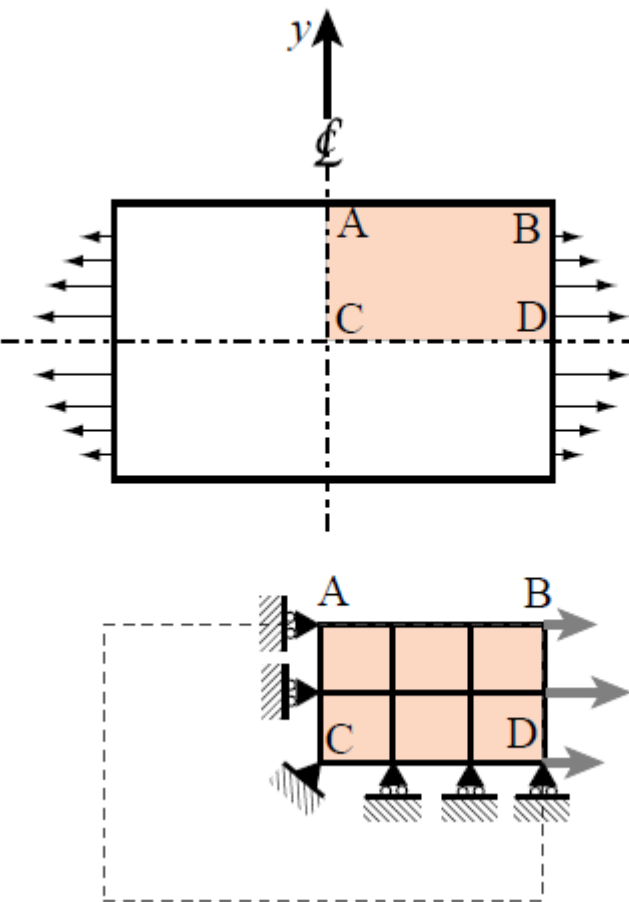
$$\sigma_{ave} = \frac{P}{A} = \frac{P}{W - 2R} = \frac{40}{40 - 5} = 1.14$$

$$\sigma_{\max} = K\sigma_{ave} = 2.8 \times 1.14 = 3.2 \text{ Pa}$$

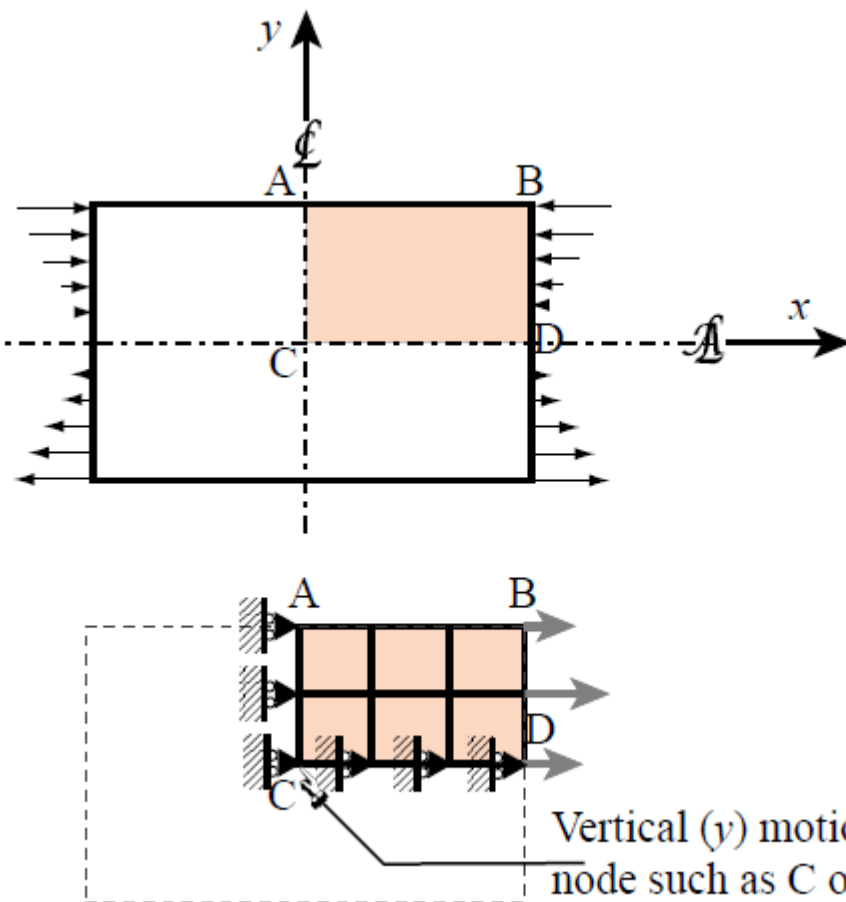
- Stiffness matrix of Turner triangle
- 2D plane stress model
 - ✓ Kirsh's problem
- Quarter model

SYMMETRY CONDITION

Symmetry condition

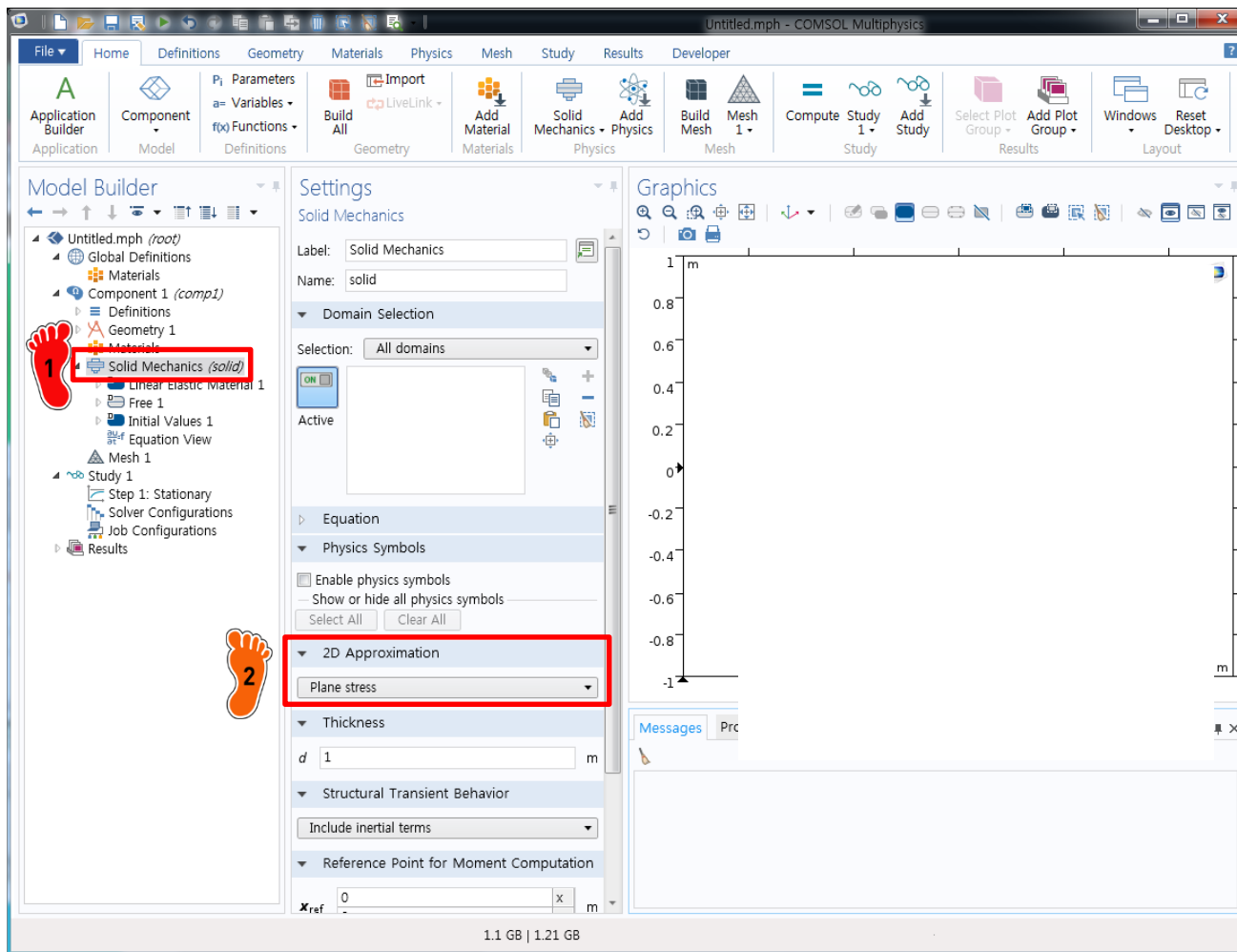


AntiSymmetry condition



Vertical (y) motion of one node such as C or D may be constrained to suppress y -RBM

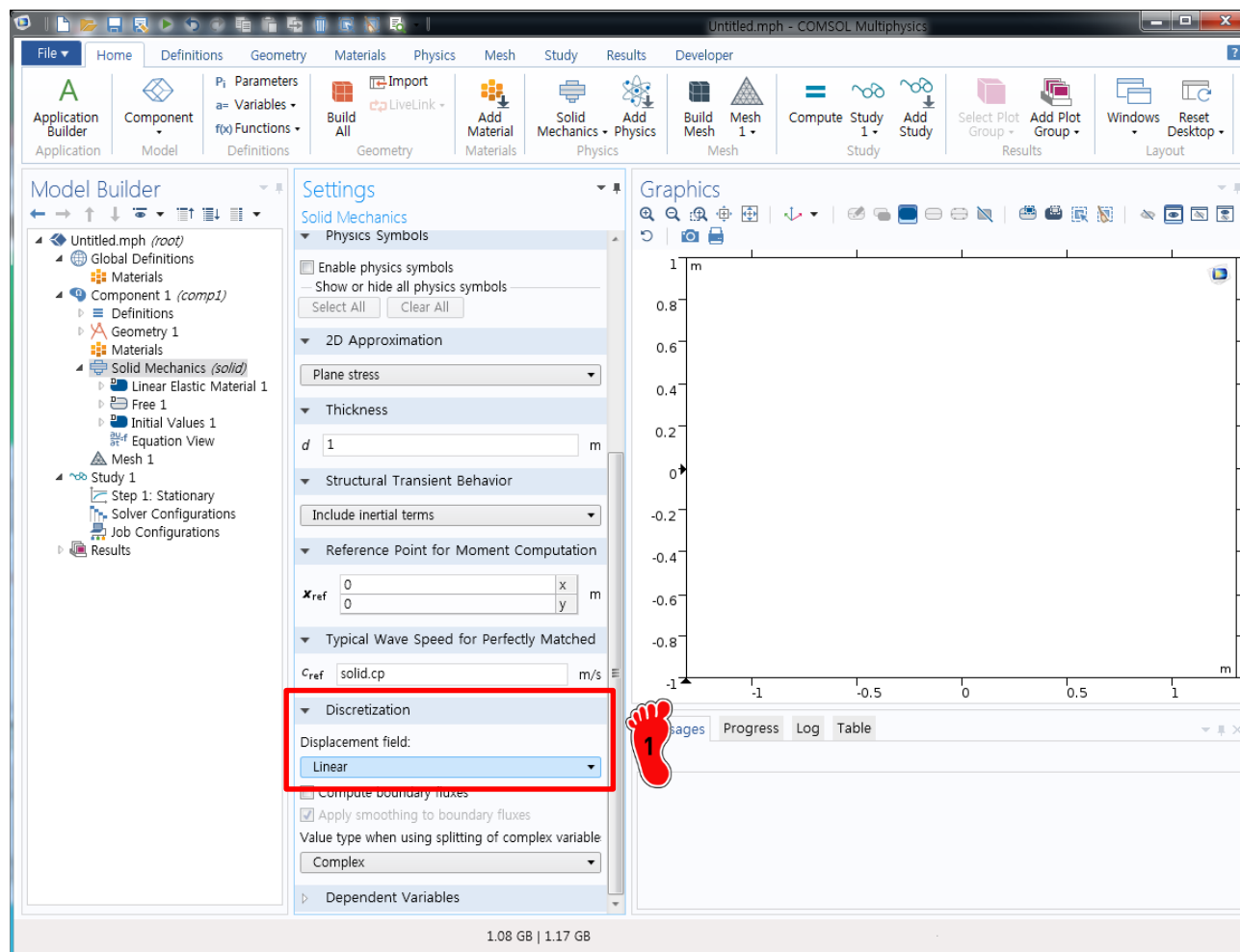
PLANE STRESS



1 Solid Mechanics 클릭

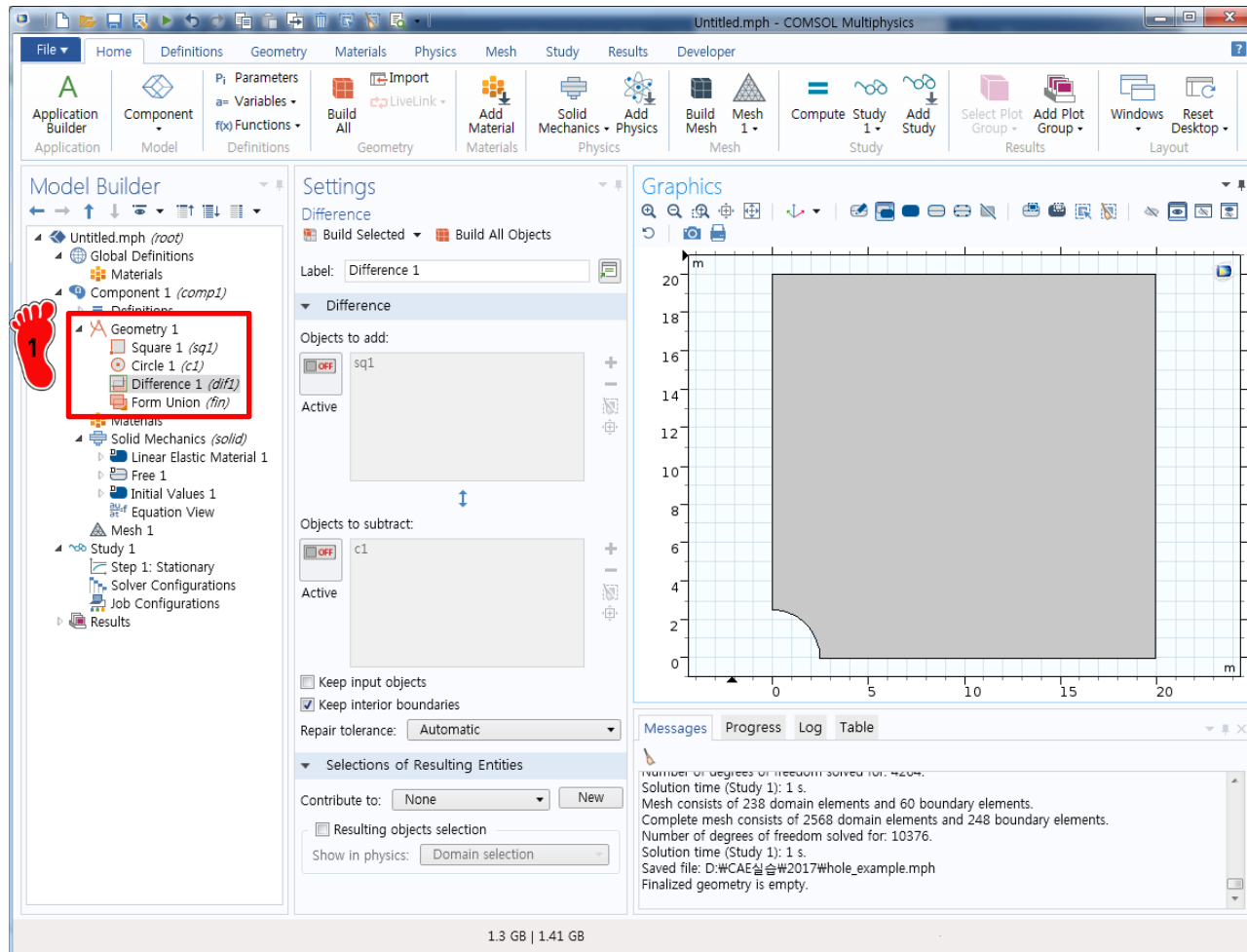
2 2D Approximation
Plane stress 로 변경

DISCRETIZATION



Discretization
Displacement field를
Linear로 변경

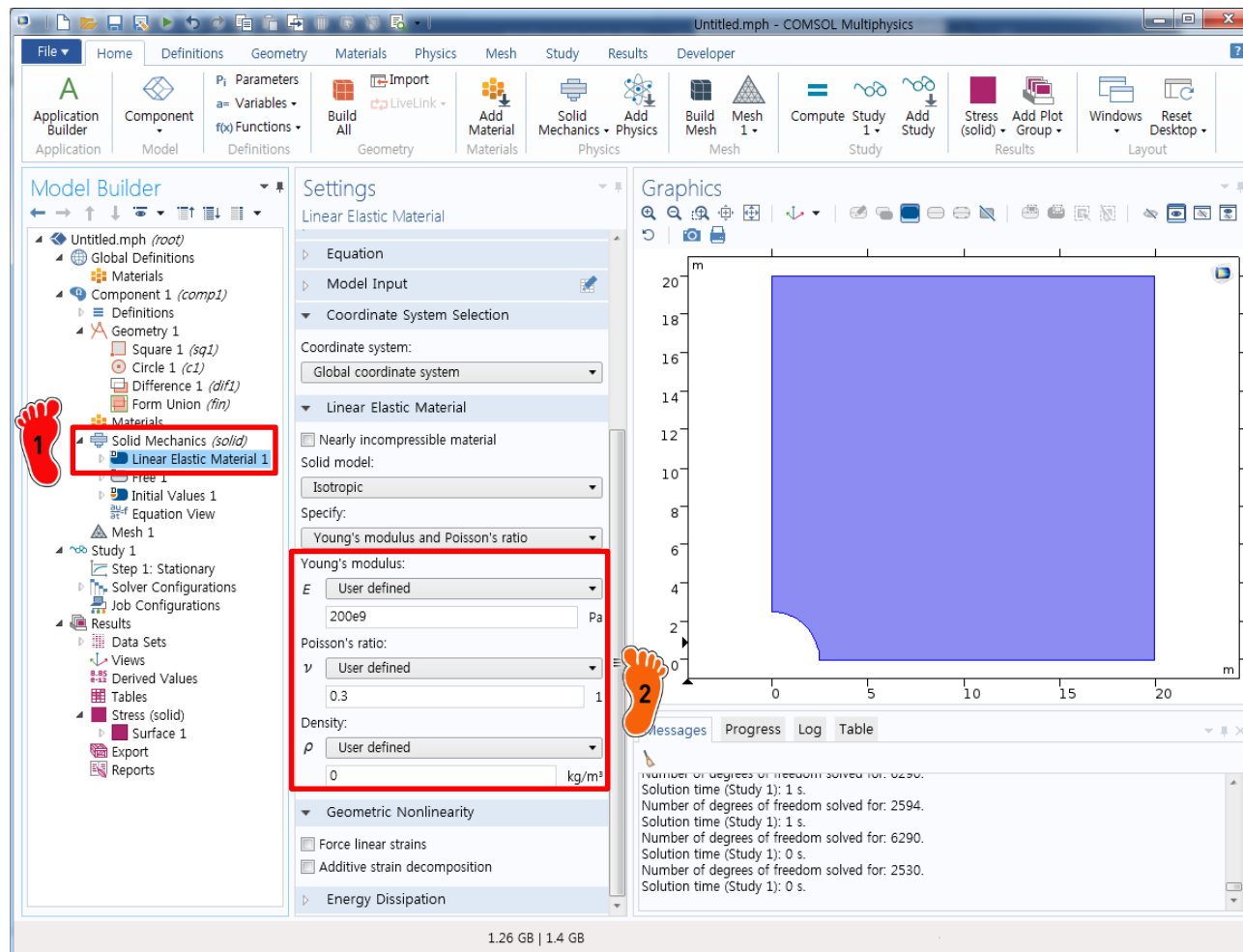
GEOMETRY CREATION



1/4 모델 기하형상 생성

길이 20의 정사각형에서
반경 2.5인 원을 뺀

MATERIAL PROPERTY



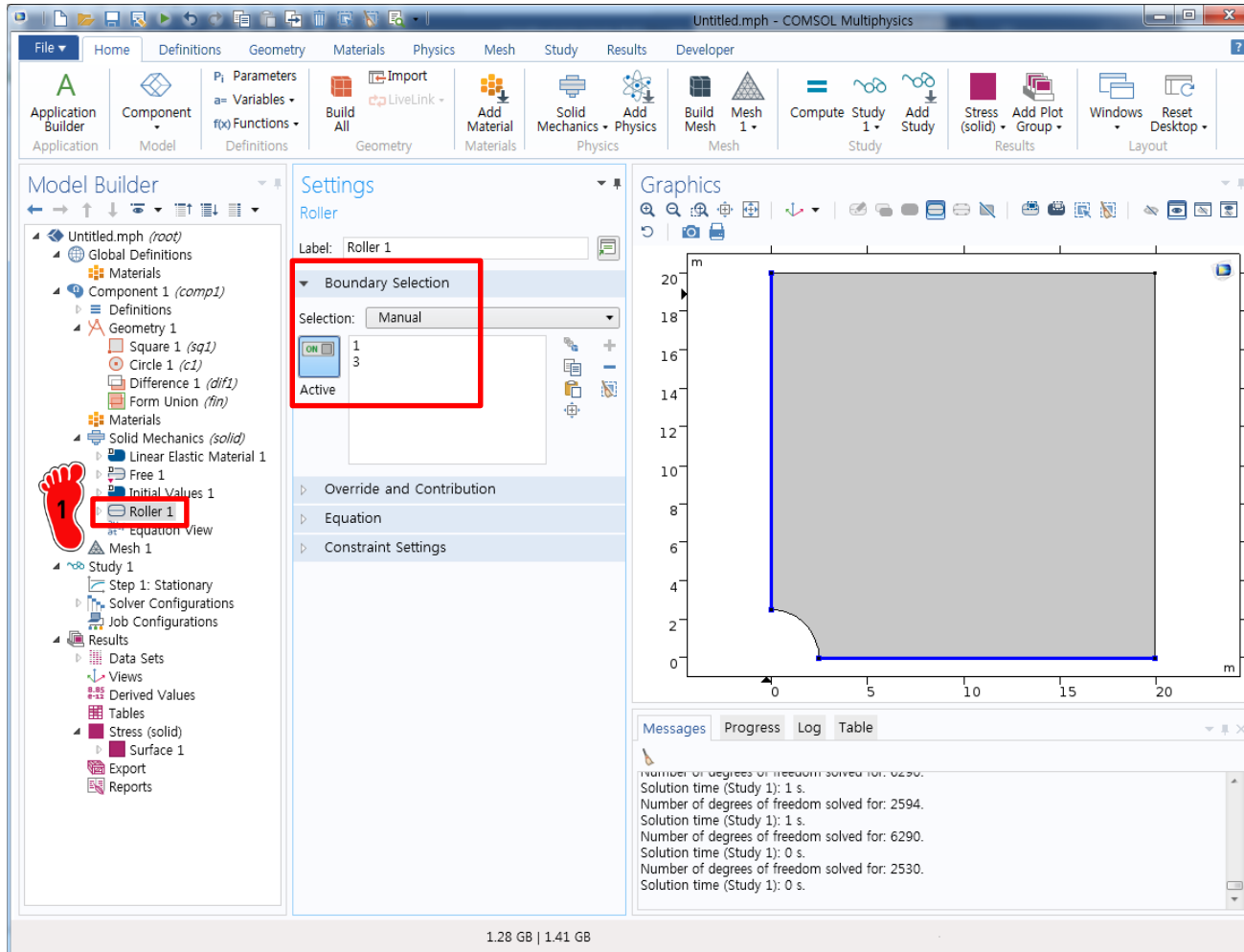
1 Linear Elastic Material 클릭

2 E: 200e9
mu: 0.3
rho : 0

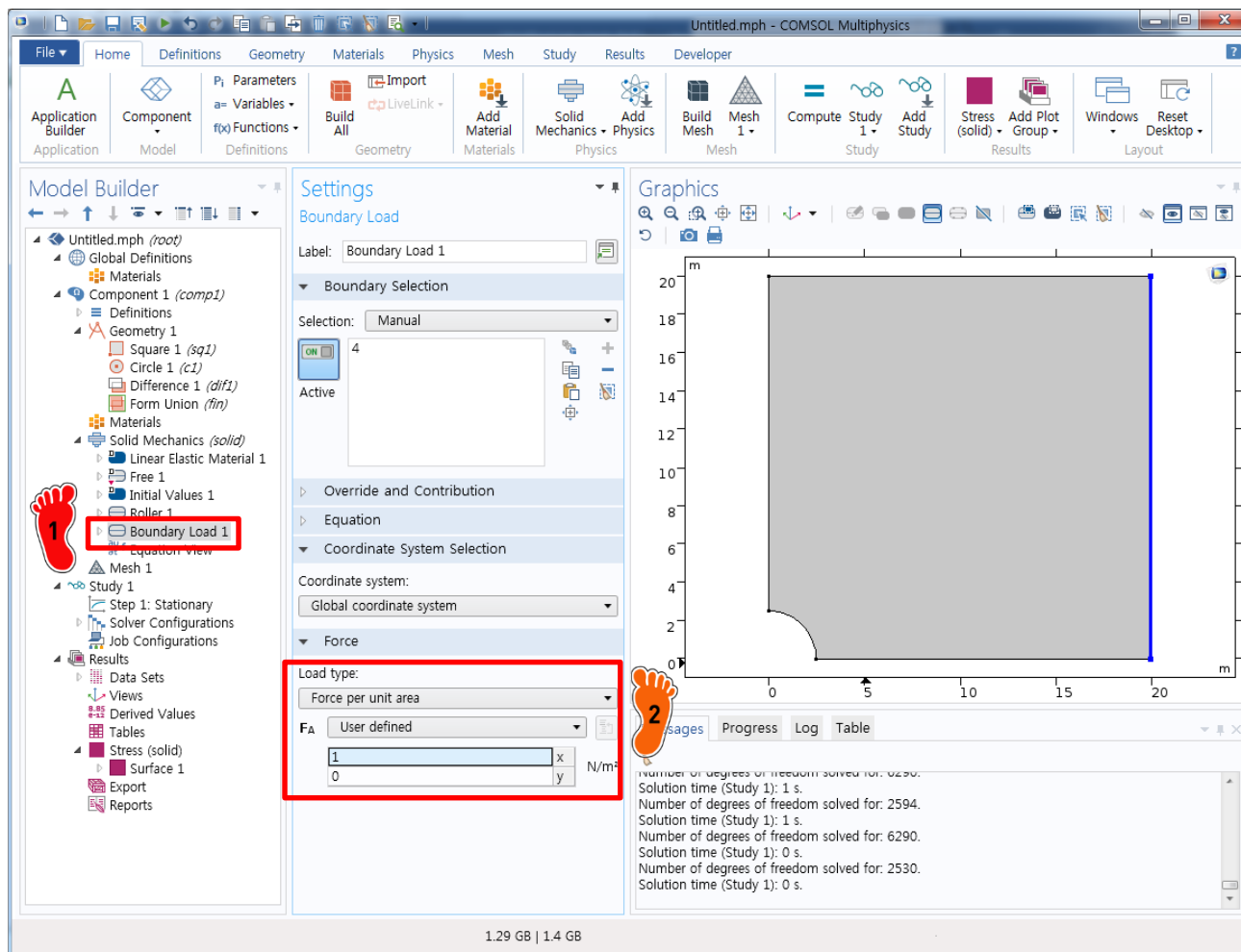
BOUNDARY CONDITION



사각형 왼쪽과 아래 변에
Roller 조건으로 입력



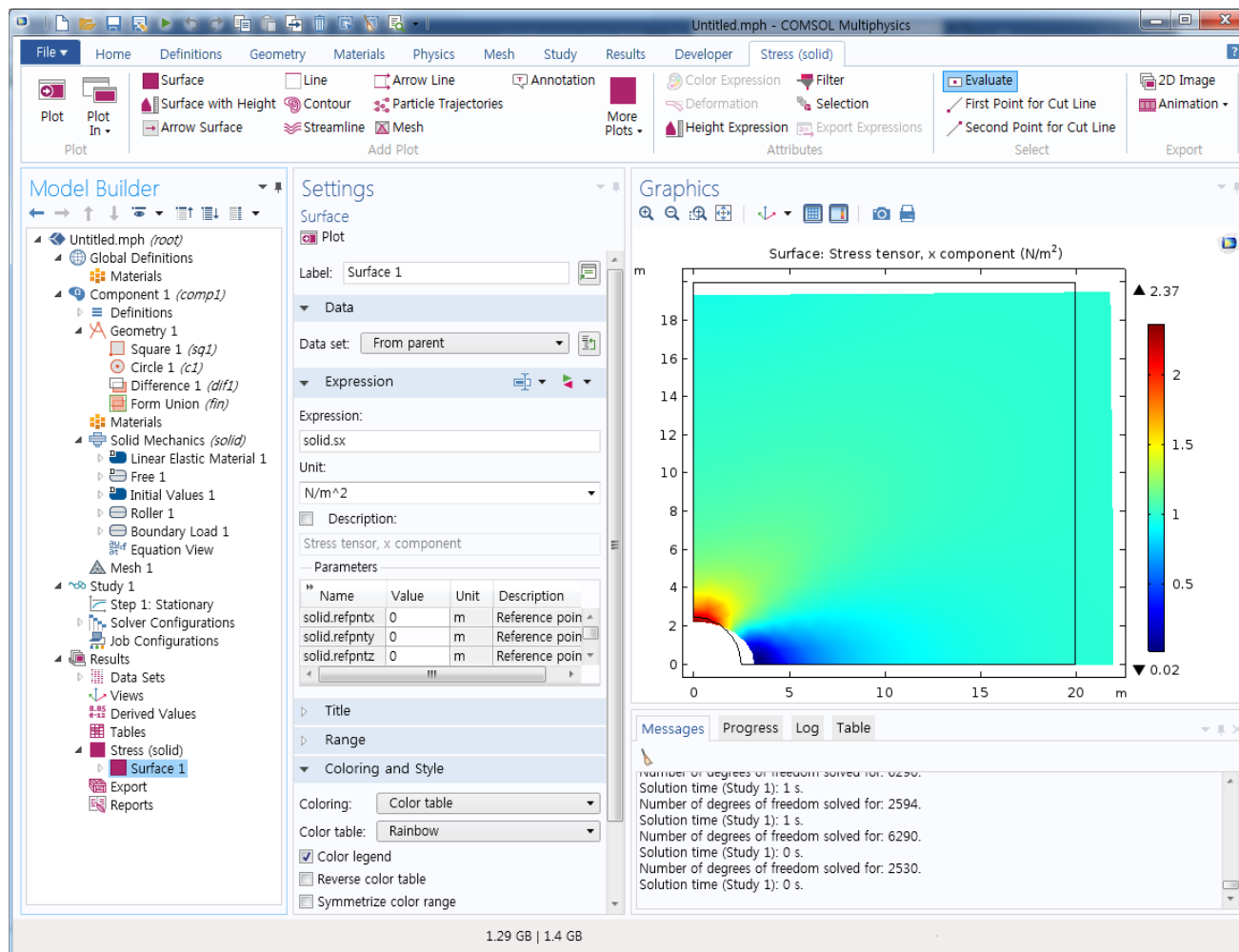
LOADING CONDITION



1 Boundary Load 생성

2 오른쪽 변에 x 방향
1 입력 (단위는 N/m²임)

RESULT



Mesh 생성 후 해석
Full model결과와 비교

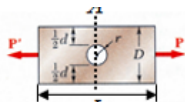
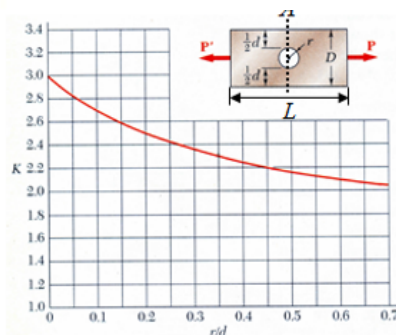
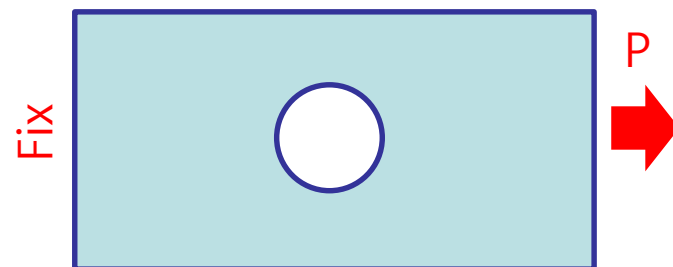
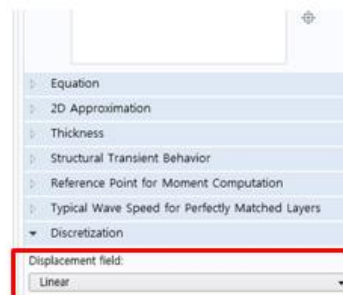
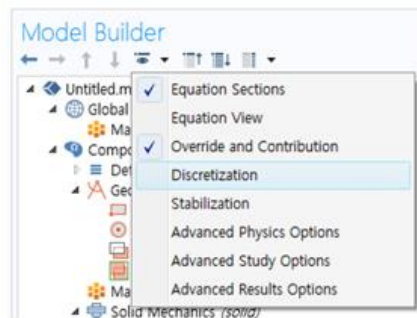
응력, 자유도 수 확인

ASSIGNMENT

2. [Stress concentration] The following flat bar with hole has a thickness of 1 m. (left side is fixed. Set the element type following discretization option.)

2016년 기말고사 2번 문제(40점)

※ Full Model 해석조건



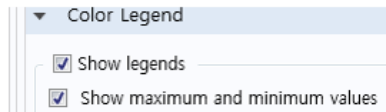
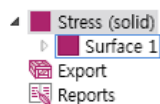
$$E = 200 \text{ GPa}, \nu = 0.33$$

$$L = 5 \text{ m}, D = 2 \text{ m}, P = 200 \text{ N}$$

$$\sigma_{ave} = \frac{P}{A}, K = \frac{\sigma_{max}}{\sigma_{ave}}$$

r [m]	0.5	0.4	0.3	0.2
r/d	0.5	0.333	0.214	0.125
K	2.18	2.30	2.46	2.62

- (1) Compute the maximum normal stress(solid.sx) with $r = 0.5 \text{ m}$. Check the stress by mesh dependency applying free triangular and quad elements(linear). Plot the graph as d.o.f vs stress changing mesh size with two cases.(mesh option : normal ~ extremely fine) (15 pts)



- (2) Compute the value of K changing the radius(0.2 ~ 0.5 m). Compare the value of K on table with computing result from FEM. (mesh option : quad & extremely fine) (15 pts)
- (3) Construct the quarter model and check the maximum stress with $r = 0.5 \text{ m}$. Compare the quarter model with full model. (10 pts)