

# Problem Formulation Process (1)

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- Step 1: Project/Problem Statement
  - Is the project goal clear?
  - descriptive statement for the project/ problem
  - overall *objectives* of the project and the *requirements* to be met
- Step 2: Data and Information Collection
  - Is all the information available to solve the problem?
  - Performance requirements, resource limits, cost of raw materials
  - Identification of analysis procedures and tools
  - project statement is vague, and assumptions about modeling of the problem need to be made in order to formulate and solve it

# Problem Formulation Process (2)

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- Step 3: Identification/Definition of Design Variables
  - What are these variables? How do I identify them?
  - identify a set of variables that describe the system, called the *design variables*
  - should be independent of each other, minimum number
  - As many independent parameters as possible should be designated as design variables at the problem formulation phase
- Step 4: Optimization Criterion
  - How do I know that my design is the best?
  - must be a scalar function whose numerical value can be obtained once a design is specified (*function of the design variable vector*)
  - *maximized* or *minimized* depending on problem requirements
  - criterion that is to be minimized is usually called a *cost function* in engineering literature

# Problem Formulation Process (3)

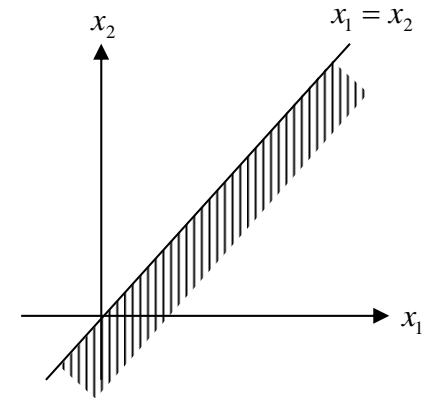
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- Step 5: Formulation of Constraints
  - What restrictions do I have on my design?
  - All restrictions placed on the design
  - identify all constraints and develop expressions for them
  - must be designed and fabricated with the given *resources* and must meet *performance requirements*

# Problem Formulation Steps

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- Identification of *design variables*
  - Parameters chosen to describe the design
  - Independent of each other, minimum number
- Identification of an *objective (cost) functions*
  - Criterion to compare various designs
  - As a function of the design variables
  - Single/Multi-objective
- Identification of all *design constraints*
  - All restrictions placed on a design
  - Feasible/Infeasible
  - Explicit/Implicit, Linear/Nonlinear, Equality/Inequality



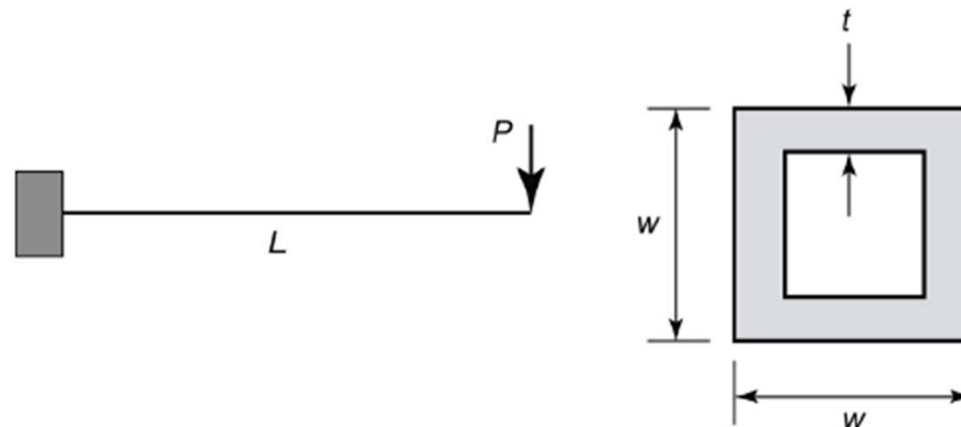
# DESIGN OF A CANTILEVER BEAM

- Step 1: Problem Statement

Cantilever beams are used in many practical applications in civil, mechanical, and aerospace engineering. To illustrate the step of problem description, we consider the design of a hollow square-cross-section *cantilever beam* to support a load of 20 kN at its end. The beam, made of steel, is 2 m long, as shown in Fig. 2.1. The failure conditions for the beam are as follows: (1) the material should not fail under the action of the load, and (2) the deflection of the free end should be no more than 1 cm. The width-to-thickness ratio for the beam should be no more than 8 to avoid local buckling of the walls. A *minimum-mass* beam is desired. The width and thickness of the beam must be within the following limits:

$$60 \leq \text{width} \leq 300 \text{ mm} \quad (\text{a})$$

$$3 \leq \text{thickness} \leq 15 \text{ mm} \quad (\text{b})$$



# DESIGN OF A CANTILEVER BEAM

- Step 2: Data and Information Collection

Notation	Data	
		$A = w^2 - (w - 2t)^2 = 4t(w - t), \text{ mm}^2$
$A$	Cross-sectional area, $\text{mm}^2$	
$E$	Modulus of elasticity of steel, $21 \times 10^4 \text{ N/mm}^2$	$I = \frac{1}{12}w \times w^3 - \frac{1}{12}(w - 2t) \times (w - 2t)^3 = \frac{1}{12}w^4 - \frac{1}{12}(w - 2t)^4, \text{ mm}^4$
$G$	Shear modulus of steel, $8 \times 10^4 \text{ N/mm}^2$	
$I$	Moment of inertia of the cross-section, $\text{mm}^4$	$Q = \frac{1}{2}w^2 \times \frac{w}{4} - \frac{1}{2}(w - 2t)^2 \times \frac{(w - 2t)}{4} = \frac{1}{8}w^3 - \frac{1}{8}(w - 2t)^3, \text{ mm}^3$
$L$	Length of the member, 2000 mm	
$M$	Bending moment, $\text{N/mm}$	$M = PL, \text{ N/mm}$
$P$	Load at the free end, 20,000 N	$V = P, \text{ N}$
$Q$	Moment about the neutral axis of the area above the neutral axis, $\text{mm}^3$	
$q$	Vertical deflection of the free end, mm	$\sigma = \frac{Mw}{2I}, \text{ N/mm}^2$
$q_a$	Allowable vertical deflection of the free end, 10 mm	
$V$	Shear force, N	$\tau = \frac{VQ}{2It}, \text{ N/mm}^2$
$w$	Width (depth) of the section, mm	
$t$	Wall thickness, mm	$q = \frac{PL^3}{3EI}, \text{ mm}$
$\sigma$	Bending stress, $\text{N/mm}^2$	
$\sigma_a$	Allowable bending stress, 165 $\text{N/mm}^2$	
$\tau$	Shear stress, $\text{N/mm}^2$	$Q = \int_A y dA : \text{ first moment of area } \rightarrow \tau = \frac{V(x)Q(y)}{Ib(y)}$
$\tau_a$	Allowable shear stress, 90 $\text{N/mm}^2$	

# DESIGN OF A CANTILEVER BEAM

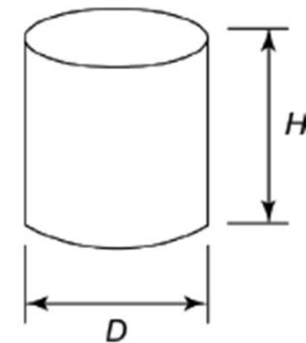
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- Step 3: Definition of Design Variables
  - $w$  = outside width (depth) of the section, mm
  - $t$  = wall thickness, mm
- Step 4: Optimization Criterion
  - Design a minimum-mass cantilever beam
  - cross-sectional area of the beam:
- Step 5: Formulation of Constraints
  - Bending stress constraint
  - Shear stress constraint
  - Deflection constraint
  - Width-thickness restriction
  - Dimension restrictions

# Design of a Can (1)

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- Step 1: Problem Statement
  - Design a can to hold at least 400ml of liquid
  - Production in billions → Minimize the manufacturing cost
  - Cost directly related to the surface area of the sheet metal
  - Minimize the sheet metal required to fabricate the can
  - Diameter of the can should be no more than 8 cm. Also, it should not be less than 3.5 cm.
  - Height of the can should be no more than 18 cm and no less than 8 cm.



- Step 2: Data and Information Collection



# Design of a Can (2)

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- Step 3: Design variables
  - Diameter of the can (cm) / Height of the can (cm)
- Step 4: Cost function
  - Total surface area of the sheet metal

$$f(D, H) = \pi DH + 2\left(\frac{\pi D^2}{4}\right)$$

- Step 5: Constraints
  - Volume:  $\left(\frac{\pi D^2}{4}\right)H \geq 400$
  - Size of the can: side/technological/sizing constraints, simple bounds, upper and lower limits

$$3.5 \leq D \leq 8; \quad 8 \leq H \leq 18$$

# Insulated Spherical Tank Design (1)

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- Step 1: Problem Statement
  - Choose insulation thickness to minimize the life-cycle cooling cost for a spherical tank
  - Cooling cost: installing and running the refrigeration equipment + installing the insulation
  - 10-yr life, 10% annual interest rate, no salvage value, tank radius:  $r$
- Step 2: Data and Information Collection
  - Capacity of the refrigeration equipment (annual heat gain)

$$G = \frac{(365)(24)(\Delta T)A}{c_1 t} \text{ W} \cdot \text{hr}$$

$A = 4\pi r^2$  : surface area of the spherical tank

$\Delta T$  : average difference between the internal and external temperatures (K)

$c_1$  : thermal resistivity per unit thickness (K · m/W)

$t$  : insulation thickness (m)

# Insulated Spherical Tank Design (2)

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- Step 3: Design variable
  - Insulation thickness:  $t$  (m)
- Step 4: Cost function
  - Insulation, refrigeration equipment, operations for 10 yrs

$$f = c_2 At + c_3 G + c_4 G \left[ \underbrace{uspwf(0.1,10)}_{=6.14457} \right] \quad (\text{assuming } t \ll r)$$

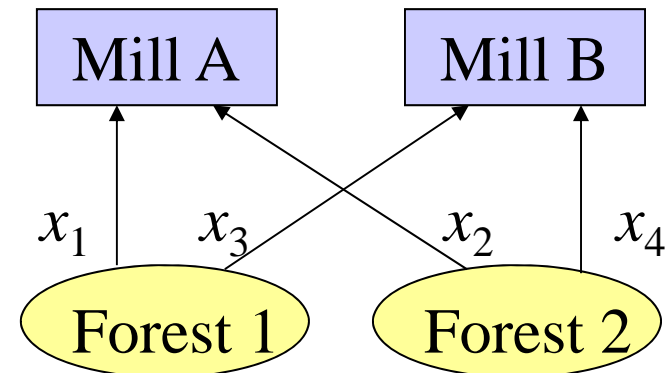
- $c_2$ : insulation cost per cubic meter (\$/m<sup>3</sup>)
  - $c_3$ : cost of the refrigeration equipment per Wh of capacity (\$/Wh)
  - $c_4$ : annual cost of running the refrigeration equipment per Wh (\$/Wh)
- Step 5: Constraints

$$t > 0 \rightarrow t \geq 0 \rightarrow t \geq t_{\min}$$

# Saw Mill Operation (1)

- Step 1: Problem Statement
  - Each forest can yield up to 200 logs/day
  - Cost to transport the logs is estimated at 15 cents/km/log
  - At least 300 logs are needed each day
  - Minimize the cost of transportation of logs each day
- Step 2: Data and Information Collection

Mill	Distance (km)		Capacity /day
	Forest 1	Forest 2	
A	24.0	20.5	240 logs
B	17.2	18.0	300 logs



# Saw Mill Operation (2)

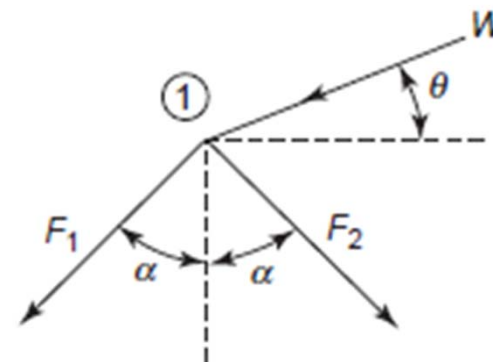
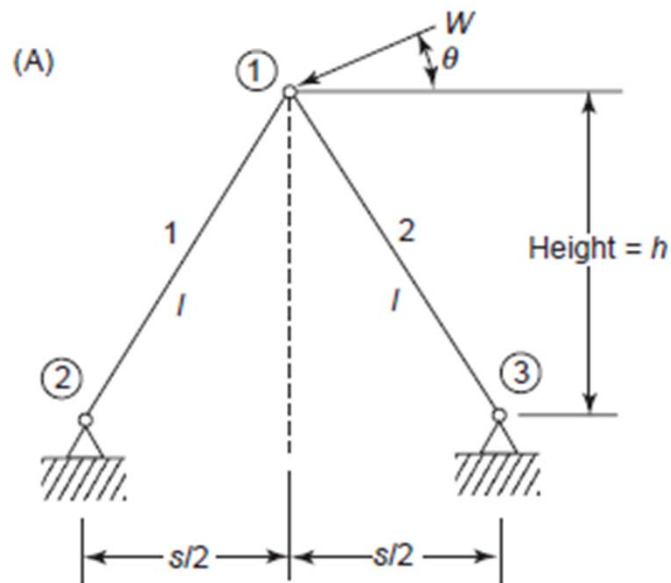
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- Step 3: Design variables :  $x_1, x_2, x_3, x_4$
- Step 4: Cost function
  - Cost of transportation of logs each day
- Step 5: Constraints
  - Mill capacities :
  - Yield of forests :

Linear Programming problem  
→ Integer Programming problem

# Two-Bar Structure (1)

- Step 1: Problem Statement
  - Design a two-bar bracket to support a force  $W$  without failure
  - Cost directly related to the size of the two bars
  - To minimize the total mass of the bracket while satisfying performance, fabrication, and space limitations



$$\begin{cases} \sum F_x = -F_1 \sin \alpha + F_2 \sin \alpha - W \cos \theta = 0 \\ \sum F_y = -F_1 \cos \alpha - F_2 \cos \alpha - W \sin \theta = 0 \end{cases}$$

$$\sin \alpha = \frac{s}{2l}, \quad \cos \alpha = \frac{h}{l}, \quad l = \sqrt{h^2 + \left(\frac{s}{2}\right)^2}$$

# Two-Bar Structure (2)

- Step 3: Design variables (hollow circular tubes)

- $x_1$ : height of the truss,  $x_2$ : span of the truss
- $x_3, x_4$ : outer/inner diameters of member 1
- $x_5, x_6$ : outer/inner diameters of member 2

$$A_1 = \frac{\pi}{4}(x_3^2 - x_4^2), A_2 = \frac{\pi}{4}(x_5^2 - x_6^2)$$

$(d_0, r) \text{ where } r = \frac{d_i}{d_0}$   
 $(d_0, d_i)$   
 $(d_0, d_i, r)?$

- Step 4: Cost function

- Minimize the mass:  $m = \rho[l(A_1 + A_2)] = \rho\sqrt{x_1^2 + (0.5x_2)^2} \frac{\pi}{4}(x_3^2 - x_4^2 + x_5^2 - x_6^2)$

- Step 5: Constraints

- stress in each member  $\leq$  material allowable stress
- Side constraints

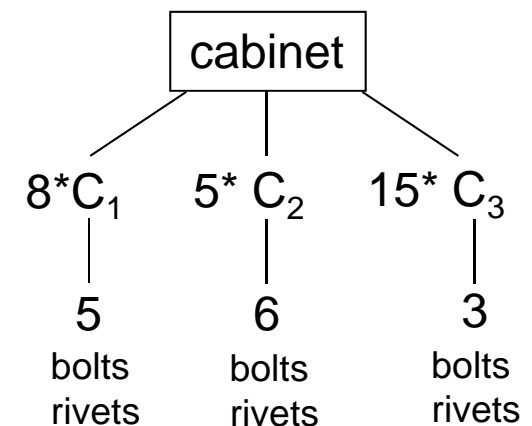
$$\left| \sigma_i = \frac{F_i}{A_i} \right| \leq \sigma_a \quad (i=1,2)$$

$$x_{il} \leq x_i \leq x_{iu} \quad (i=1,\dots,6)$$

# Design of a Cabinet

- Determine the number of components to be bolted and riveted to minimize the cost
  - Each cabinet requires  $8 \cdot C_1$ ,  $5 \cdot C_2$ ,  $15 \cdot C_3$  components
  - Assembly of  $C_1$  needs either 5 bolts or 5 rivets;  $C_2$  6 bolts or 6 rivets ;  $C_3$  3 bolts or 3 rivets
  - A total of 100 cabinets must be assembly daily
  - Bolting and riveting capacities per day are 6000 and 8000, respectively

Cost (\$)	$C_1$	$C_2$	$C_3$
bolt	0.7	1.0	0.6
rivet	0.6	0.8	1.0





# Formulation 1 (component level)

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- Design variables (for 100 cabinets)
  - $x_1/ x_3/ x_5$  = number of  $C_1/ C_2/ C_3$  to be bolted
  - $x_2/ x_4/ x_6$  = number of  $C_1/ C_2/ C_3$  to be riveted
- Cost function
- Constraints

## Formulation 2 (bolt/rivet level)

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- Design variables
  - $x_1/ x_2/ x_3$  = total number of bolts required for all  $C_1/ C_2/ C_3$
  - $x_4/ x_5/ x_6$  = total number of rivets required for all  $C_1/ C_2/ C_3$
- Cost function
- Constraints

# Formulation 3 (←Formulation 1)

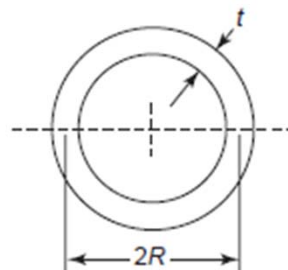
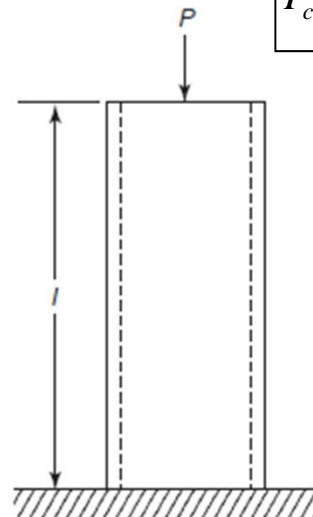
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- Design variables (for one cabinet)
  - $x_1/ x_3/ x_5$  = number of  $C_1/ C_3/ C_5$  to be bolted on one cabinet
  - $x_2/ x_4/ x_6$  = number of  $C_2/ C_4/ C_6$  to be riveted on one cabinet
- Cost function
- Constraints

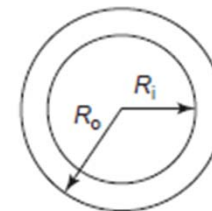
# Minimum Weight Tubular Column Design

- Step 1: Problem Statement
  - Straight columns: structural elements (street light pole, traffic light post, water tower support)
  - Design a minimum mass tubular column of length  $l$  supporting a load  $P$  w/o buckling or overstressing
- Step 2: Data and Information Collection
  - Buckling load

$$P_{cr} = \frac{\pi^2 EI}{4l^2}$$

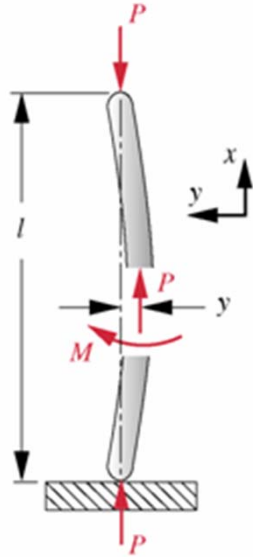


Formulation 1



Formulation 2

# Buckling of an Euler Column



$$\frac{d^2 y}{dx^2} = \frac{M}{EI} \rightarrow EI \frac{d^2 y}{dx^2} = M = -Py$$

$$\frac{d^2 y}{dx^2} + \left( \frac{P}{EI} \right) y = 0$$

$$y = c_1 \sin \left( \sqrt{\frac{P}{EI}} x \right) + c_2 \cos \left( \sqrt{\frac{P}{EI}} x \right)$$

+ boundary conditions

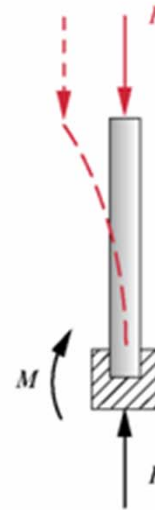
$$\rightarrow P_{cr} = \frac{\pi^2 EI}{l_{eff}^2}$$



(a) Rounded-rounded



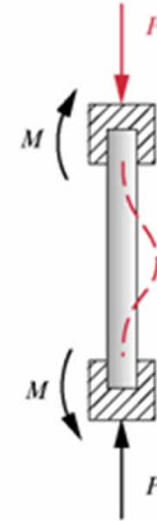
(b) Pinned-pinned



(c) Fixed-free



(d) Fixed-pinned



(e) Fixed-fixed

End Conditions	Theoretical Value	AISC* Recommends	Conservative Value
Rounded-Rounded	$l_{eff} = l$	$l_{eff} = l$	$l_{eff} = l$
Pinned-Pinned	$l_{eff} = l$	$l_{eff} = l$	$l_{eff} = l$
Fixed-Free	$l_{eff} = 2l$	$l_{eff} = 2.1l$	$l_{eff} = 2.4l$
Fixed-Pinned	$l_{eff} = 0.707l$	$l_{eff} = 0.80l$	$l_{eff} = l$
Fixed-Fixed	$l_{eff} = 0.5l$	$l_{eff} = 0.65l$	$l_{eff} = l$

# Formulation 1

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- Step 3: Design variables
  - $R$  (mean radius of column) /  $t$  (wall thickness)

- Step 4: Cost function

$$\text{mass} = \rho(lA) = 2\rho l\pi R t$$

$$\left[ \text{assuming thin wall } (R \gg t) \rightarrow A = 2\pi R t; I = \pi R^3 t \right]$$

- Step 5: Constraints

$$\begin{cases} \sigma = \frac{P}{A} = \frac{P}{2\pi R t} \leq \sigma_a \\ P \leq \frac{\pi^2 EI}{4l^2} = \frac{\pi^3 ER^3 t}{4l^2} \end{cases}$$

$$R_{\min} \leq R \leq R_{\max}; \quad t_{\min} \leq t \leq t_{\max}$$

# Formulation 2

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- Step 3: Design variables
  - $R_o$  (outer radius of column) /  $R_i$  (inner radius of column)

- Step 4: Cost function

$$\text{mass} = \rho(lA) = \pi\rho l(R_o^2 - R_i^2) \quad \left[ A = \pi(R_o^2 - R_i^2); I = \frac{\pi}{4}(R_o^4 - R_i^4) \right]$$

- Step 5: Constraints

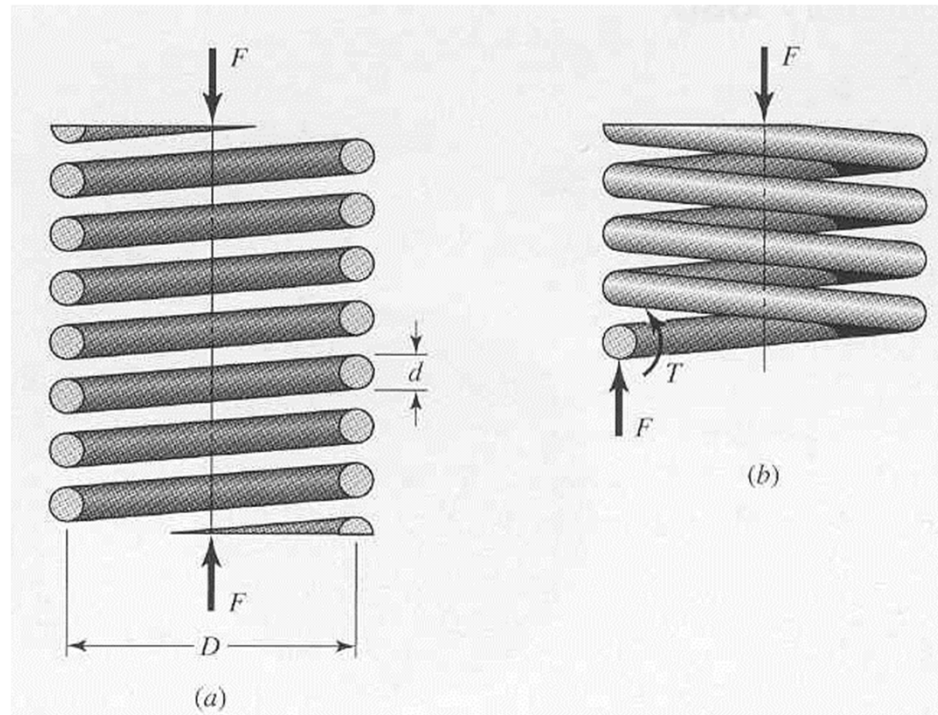
$$\begin{cases} \sigma = \frac{P}{A} = \frac{P}{\pi(R_o^2 - R_i^2)} \leq \sigma_a \\ P \leq \frac{\pi^2 EI}{4l^2} = \frac{\pi^3 E}{16l^2}(R_o^4 - R_i^4) \end{cases}$$

$$(R_o)_{\min} \leq R_o \leq (R_o)_{\max}; \quad (R_i)_{\min} \leq R_i \leq (R_i)_{\max}$$

$$R_o > R_i; \quad \underbrace{\frac{R}{t} = \frac{R_o + R_i}{2(R_o - R_i)}}_{\text{to avoid local buckling}} \leq k \quad (\text{thin-walled: } R \gg t, k \geq 20)$$

# Design of Coil Spring

- Step 1: Problem Statement
  - To design a minimum mass spring to carry a given axial load without material failure and while satisfying two performance requirement: the spring must deflect by at least  $\Delta$  (in), and the frequency of surge waves must not be less than  $\omega_0$  (Hz)





## Step 2: Data and Information Collection (1)

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- Deflection along the axis of the spring:  $\delta$  (in)
- Mean coil diameter:  $D$  (in)
- Wire diameter:  $d$  (in)
- Number of active coils:  $N$
- Gravitational constant:  $g = 386$  (in/s<sup>2</sup>)
- Frequency of surge waves:  $\omega$  (Hz)

## Step 2: Data and Information Collection (2)

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- Material property
  - Weight density:  $\gamma = 0.285 \text{ (lb/in}^3 \text{)}$
  - Shear modulus:  $G = 1.15\text{E}7 \text{ (lb/in}^2 \text{)}$
  - Mass density:  $\rho = 7.38342\text{E-}4 \text{ (lb-s}^2\text{/in}^4 \text{)}$
  - Allowable shear stress:  $\tau_a = 80000 \text{ (lb/in}^2 \text{)}$
- Other data
  - Number of inactive coils:  $Q = 2$
  - Applied load:  $P = 10 \text{ (lbs)}$
  - Minimum spring deflection:  $\Delta = 0.5 \text{ (in)}$
  - Lower limit on surge wave frequency:  $\omega_0 = 100 \text{ (Hz)}$
  - Limit on outer diameter of the coil:  $D_0 = 1.5 \text{ (in)}$

# Design equations for the spring (1)

- Load-deflection

$$U = \underbrace{\frac{T^2 L}{2GJ}}_{\text{torsion}} + \underbrace{\frac{F^2 L}{2GA}}_{\text{shear}} = \frac{F^2 (D/2)^2 \pi D (N+Q)}{2G (\pi d^4/32)} + \frac{F^2 \pi D (N+Q)}{2G (\pi d^2/4)} = \frac{4F^2 D^3 (N+Q)}{d^4 G} + \frac{2F^2 D (N+Q)}{d^2 G}$$

$$\xrightarrow{\text{by Castigliano's theorem}} \delta = \frac{\partial U}{\partial F} = \frac{8FD^3 (N+Q)}{d^4 G} + \frac{4FD (N+Q)}{d^2 G}$$

$$\xrightarrow{C = \frac{D}{d}} \delta = \frac{8FD^3 (N+Q)}{d^4 G} \left( 1 + \frac{1}{2C^2} \right) \approx \frac{8FD^3 (N+Q)}{d^4 G}$$

- Shear stress

$$\tau_{\max} = \frac{Tr}{J} + \frac{F}{A} = \frac{F(D/2)(d/2)}{(\pi d^4/32)} + \frac{F}{\pi d^2/4} = \frac{8FD}{\pi d^3} + \frac{4F}{\pi d^2}$$

$$= \frac{8FD + 4Fd}{\pi d^3} = \left( 1 + \frac{d}{2D} \right) \frac{8FD}{\pi d^3} = K_s \frac{8FD}{\pi d^3}$$

$$\begin{cases} K_s = 1 + 0.5 \frac{d}{D} \\ K_w = \frac{4D-1}{4(D-d)} + \frac{0.615d}{D} \end{cases}$$

# Design equations for the spring (2)

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- Frequency of surge waves

$$\frac{\partial^2 u}{\partial y^2} = \frac{W}{kgl^2} \frac{\partial^2 u}{\partial t^2}, \quad \text{B.C. } u(0,t) = 0 \text{ and } u(l,t) = 0$$

$$W = AL\gamma = \left( \frac{\pi d^2}{4} \right) (\pi DN) \gamma = \frac{\pi^2 d^2 DN \gamma}{4}$$

$$\omega_m = m\pi \sqrt{\frac{kg}{W}}, \quad \text{fundamental frequency } (m = 1)$$

$$f_1 = \frac{\omega_1}{2\pi} = \frac{1}{2} \sqrt{\frac{kg}{W}} = \frac{2}{\pi N} \frac{d}{D^2} \sqrt{\frac{Gg}{32\gamma}} = \frac{d}{2\pi ND^2} \sqrt{\frac{G}{2\rho}}$$

# Problem Formulation

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- Step 3: Identification of design variables

- Wire diameter:  $d$
- Mean coil diameter:  $D$
- Number of active coils:  $N$

- Step 4: Identification of an objective function

- Mass

$$m = \rho AL = \rho \left( \frac{\pi d^2}{4} \right) \pi D (N + Q) = \frac{\pi^2 \rho d^2 D (N + Q)}{4}$$

- Step 5: Identification of constraints

- Deflection:  $\delta \geq \Delta$
- Shear stress:  $\tau \leq \tau_a$
- Frequency of surge waves:  $\omega \geq \omega_0$
- Diameter:  $D + d \leq D_0$
- Side constraints:  $d_{\min} \leq d \leq d_{\max}, D_{\min} \leq D \leq D_{\max}, N_{\min} \leq N \leq N_{\max}$

# Mathematical Formulation

$$\begin{aligned}
 &\underset{d,D,N}{\text{Minimize}} \quad m = \frac{\pi^2 \rho d^2 D (N + Q)}{4} \\
 &\text{subject to} \quad \frac{8FD^3(N+Q)}{d^4G} \geq \Delta \rightarrow 1 - \frac{8FD^3(N+Q)}{d^4G\Delta} \leq 0 \\
 &\quad \left[ \frac{4D-d}{4(D-d)} + \frac{0.615d}{D} \right] \frac{8FD}{\pi d^3} \leq \tau_a \rightarrow \left[ \frac{4D-d}{4(D-d)} + \frac{0.615d}{D} \right] \frac{8FD}{\pi d^3 \tau_a} - 1 \leq 0 \\
 &\quad \frac{d}{2\pi ND^2} \sqrt{\frac{G}{2\rho}} \geq \omega_0 \rightarrow 1 - \frac{d}{2\pi ND^2 \omega_0} \sqrt{\frac{G}{2\rho}} \leq 0 \\
 &\quad D + d \leq D_0 \rightarrow \frac{D+d}{D_0} - 1 \leq 0 \\
 &\quad d_{\min} \leq d \leq d_{\max} \\
 &\quad D_{\min} \leq D \leq D_{\max} \\
 &\quad N_{\min} \leq N \leq N_{\max}
 \end{aligned}$$

# Symmetric Three-Bar Truss (1)

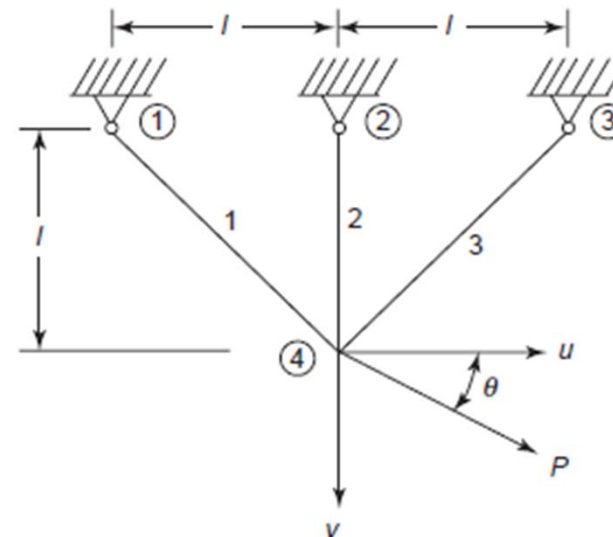
- Step 1: Problem Statement
  - Design for minimum volume to support a force  $P$
  - Consideration of member crushing, member buckling, failure by excessive deflection of node 4, failure by resonance
- Step 2: Data and Information Collection
  - Equilibrium equations  $\rightarrow$  displacements  $\rightarrow$  forces carried by the members of the truss  $\rightarrow$  stress

$$\sigma_1 = \frac{1}{\sqrt{2}} \left[ \frac{P_u}{A_1} + \left( \frac{P_v}{A_1 + \sqrt{2}A_2} \right) \right]$$

$$\sigma_2 = \frac{\sqrt{2}P_v}{(A_1 + \sqrt{2}A_2)}$$

$$\sigma_3 = \frac{1}{\sqrt{2}} \left[ -\frac{P_u}{A_1} + \left( \frac{P_v}{A_1 + \sqrt{2}A_2} \right) \right]$$

$$\omega = \frac{3EA_1}{\rho l (4A_1 + \sqrt{2}A_2)}$$



# Symmetric Three-Bar Truss (2)

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- Step 3: Design variables
  - $A_1$ : cross-sectional area of material for members 1 and 3
  - $A_2$ : cross-sectional area of material for members 2
- Step 4: Cost function
  - Material volume:  $V = l(2\sqrt{2}A_1 + A_2)$
- Step 5: Constraints
  - stress:  $\sigma_1 \leq \sigma_a, \sigma_2 \leq \sigma_a \leftarrow \sigma_1 > \sigma_3$
  - displacement:  $u \leq \Delta_u, v \leq \Delta_v$
  - natural frequency:  $f_0 \geq (2\pi\omega_0)^2$
  - buckling:  $-F_i \leq \frac{\pi^2 EI}{l_i^2}, \quad I = \beta A^2$
  - side:  $A_1, A_2 \geq A_{\min}$



# Standard Design Optimization Model

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Find an  $n$ -vector  $\mathbf{x} = (x_1, \dots, x_n)$  of design variables  
to minimize a cost function

$$f(\mathbf{x}) = f(x_1, \dots, x_n)$$

subject to

$$\left\{ \begin{array}{l} \text{the } p \text{ equality constraints} \\ h_j(\mathbf{x}) = h_j(x_1, \dots, x_n) = 0; \quad j = 1, \dots, p \\ \text{and the } m \text{ inequality constraints} \\ g_i(\mathbf{x}) = g_i(x_1, \dots, x_n) \leq 0; \quad i = 1, \dots, m \end{array} \right.$$

bounds on design variables:

$$x_i \geq 0 \quad \text{or} \quad x_{il} \leq x_i \leq x_{iu}; \quad i = 1, \dots, n$$

# Observations (1)

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- Functions must depend on design variables.
- Number of independent equality constraints:  $p \leq n$ 
  - $p > n$  : overdetermined system of equations
    - redundant equality constraints
    - Inconsistent formulation
  - $p = n$  : no optimization is necessary
- Inequality constraints written as “ $\leq 0$ ”
  - No restriction on the number of inequality constraints
- Scaling effect
  - optimum design does not change. optimum cost function value, however, changes.
    - cost function by a positive constant
    - Inequality constraints by a positive constant
    - equality constraints by any constants

# Observations (2)

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- Maximization problem treatment

$$f(\mathbf{x}) = -F(\mathbf{x})$$

- “ $\geq$  type” constraints

$$G_j(\mathbf{x}) \geq 0 \rightarrow g_j(\mathbf{x}) = -G_j(\mathbf{x}) \leq 0$$

- Discrete and Integer design variables

- Approach 1

- Solve the problem assuming continuous DVs
- Assign nearest discrete/integer values
- Check feasibility  $\leftarrow$  numerous combinations

- Approach 2 (adaptive numerical optimization)

- Obtain optimum solution with continuous DVs
- Assign only DVs close to their discrete/integer values
- Optimize the problem until all DVs have proper values

# Observations (3)

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- Feasible set: collection of all feasible designs

$$S = \{\mathbf{x} | h_j(\mathbf{x}) = 0; j = 1, \dots, p; \quad g_i(\mathbf{x}) \leq 0; i = 1, \dots, m\}$$

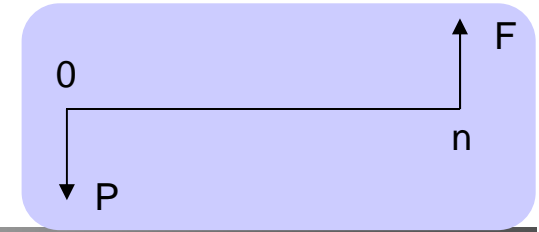
- Inequality constraint:

$$g_i(\mathbf{x}) \leq 0 \rightarrow \begin{cases} \text{active/tight/binding} : g_i(\mathbf{x}^*) = 0 \\ \text{inactive} : g_i(\mathbf{x}^*) < 0 \\ \text{violated} : g_i(\mathbf{x}^*) > 0 \end{cases}$$

# Supplements

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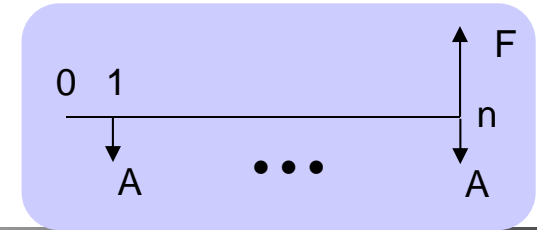
# Interest Formula (1)



- Three components of cash flow
  - Present(P), Future(F), Annuity(A)
  - 6 relationships
    - $n$ : number of interest periods, e.g., months, years
    - $i$ : return per dollar per period, e.g., annual interest rate
- F/P, P/F
  - Given P, Find F
    - Single payment compound amount factor (일회 지불 복리계수)
  - Given F, Find P
    - Single payment present worth factor (일회 지불 현가계수)

$$\begin{aligned} P &\xrightarrow{iP} (1+i)P \xrightarrow{i(1+i)P} (1+i)^2 P \longrightarrow \dots \\ F &= (1+i)^n P = spcaf(i, n)P \\ P &= (1+i)^{-n} F = sppwf(i, n)F \end{aligned}$$

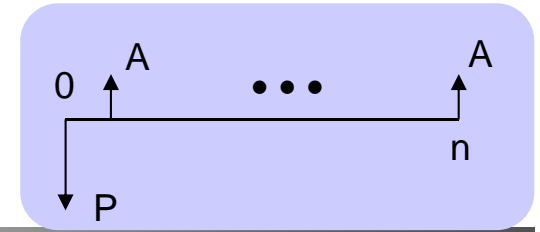
# Interest Formula (2)



- F/A, A/F
  - Given A, Find F
    - Uniform series compound amount factor (연금 복리 계수)
  - Given F, Find A
    - Sinking fund deposit factor (감채 기금 계수)

$$\begin{aligned}
 (1+i)^{n-1}A + (1+i)^{n-2}A + \cdots + A &= F \\
 \frac{(1+i)^{n-1} [1 - (1+i)^{-n}]}{1 - (1+i)^{-1}} &= \frac{(1+i)^{n-1} [1 - (1+i)^{-n}]}{\frac{i}{1+i}} = \frac{1}{i} [(1+i)^n - 1] \\
 F &= \frac{1}{i} [(1+i)^n - 1] A = uscaf(i, n) A \\
 A &= \frac{i}{[(1+i)^n - 1]} F = sfd f(i, n) F
 \end{aligned}$$

## Interest Formula (3)



- P/A, A/P
  - Given A, Find P
    - Uniform series present worth factor (연금 현가 계수)
  - Given P, Find A
    - Capital recovery factor (자본 회수 계수)

$$F = \frac{1}{i}[(1+i)^n - 1]A = (1+i)^n P \rightarrow P = \frac{1}{i}[1 - (1+i)^{-n}]A = uspwf(i, n)A$$
$$A = \frac{i}{[1 - (1+i)^{-n}]}P = crf(i, n)P$$



# Summary: Interest Formula

To find	Given	Multiply by	Description
$S_n$	$P$	$spcf(i, n) = (1+i)^n$	Single payment compound amount factor
$P$	$S_n$	$sppwf(i, n) = (1+i)^{-n}$	Single payment present worth factor
$S_n$	$R$	$uscfa(i, n) = \frac{1}{i}[(1+i)^n - 1]$	Uniform series compound amount factor
$R$	$S_n$	$sfd(i, n) = \frac{i}{[(1+i)^n - 1]}$	Sinking fund deposit factor
$P$	$R$	$uspwf(i, n) = \frac{1}{i}[1 - (1+i)^{-n}]$	Uniform series present worth factor
$R$	$P$	$crf(i, n) = \frac{i}{[1 - (1+i)^{-n}]}$	Capital recovery factor